

Geotechnical Completion Report

99 ESCOTTS ROAD STAGE 1, TUAKAU

For

HUGHES DEVELOPMENTS LIMITED

27 June 2019 Ref No: J00779

Hughes Developments Limited C/- CivilPlan Consultants Limited PO Box 97796 Auckland 2241

Attention: Mr R Pitkethley

Dear Ryan

RE: Geotechnical Completion Report for Residential Subdivision at Stage 1 – 99 Escotts Road, Tuakau

This report presents all supporting geotechnical data and our Suitability Statement in relation to land development works undertaken at the above location.

It has been prepared in accordance with instructions received from CivilPlan Consultants Limited and forms part of the documentation required by Auckland Council to achieve certification under Section 224(c) of the Resource Management Act.

If you have any queries or you require any further clarification on any aspects of this report, please do not hesitate to contact the undersigned.

For and on behalf of Lander Geotechnical Consultants Limited

S.G. Lander

Principal Geotechnical Engineer CMEngNZ, CPEng., IntPE(NZ)

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1 INTRODUCTION AND DESCRIPTION OF SUBDIVISION

This Geotechnical Completion Report has been prepared for Hughes Developments Limited as part of the documentation required to be submitted to the Waikato District Council following residential subdivisional development.

It contains our Suitability Statement, relevant test data and the CivilPlan Consultants Limited as-built plan set relating to Stage 1 of the 99 Escotts Road Residential Subdivision as follows:

Table 1: CivilPlan Consultants Limited As-Built Drawings

Title	Reference No.	Date
Finished Contours Asbuilt Plan	2011-01-AB200	May 2019
Underfill Drains Asbuilt Plan (3 sheets)	2011-01-AB205 to AB207	May 2019
Isopach Asbuilt Plan (2 sheets)	2011-01-AB220 & AB225	May 2019
Retaining Wall Asbuilt Plan (2 sheets)	2011-01-AB270 & AB271	May 2019
Slope Analysis Asbuilt Plan (2 sheets)	2011-01-AB290	May 2019

This report covers the construction period September 2018 to June 2019. It is intended to be used for certification purposes as follows:

- 54 residential lots numbered 1 to 42 and 55 to 66;
- 4 new roads named Road 1, Road 2 (part), Road 3, and Road 4;
- 1 drainage reserve numbered as lot 300. This lot contains a stormwater detention tank and an overland flow path.

This stage of the subdivision is located at 99 Escotts Road and as can be seen on the fill as-built plan, all of the lots have been partly or totally affected by filling, to a maximum depth of approximately 6.5 metres.

2 RELATED REPORTS

A Geotechnical Investigation Report on the subject land was prepared by this Consultancy, reference J00779-Rev1, dated 28 March 2018. In addition, a geotechnical design report (dated 9 May 2018, reference J00779) was prepared for retaining walls 1 to 3. The conclusions and recommendations of these reports have been reviewed during the preparation of this document.



3 EARTHWORKS OPERATIONS

3.1 Plant

The main items of plant used by the Contractor, Twomey Construction Limited were:

- Excavators (various sizes);
- 2x Twin Motorscrapers;
- 1x 4WD Padfoot Compactor;
- 1x Articulated Dump Truck;
- 1x Smooth Drum Roller;
- 1 Bulldozer;
- 1x Tractor with Disc Ploughs.

3.2 Construction Programme

Earthworks operations for this stage commenced in September 2018 with the stripping of topsoil and the mucking out (i.e. removal of organics and soft sediments) of the gully within in the southern portion of Stage 1. An underfill drain was installed in the invert of the gully (its construction is discussed in Section 5.5) and a 0.6m-thick rotten rock mattress was placed in the invert of the gully (over the underfill drain) to elevate the clay fill above the groundwater table and provide a hard base to compact upon. Most of the gully was subsequently filled with compacted clay and a silt retention pond was constructed.

In early October 2018, bulk earthworks commenced in the central portions of Stage 1 and installation of public service lines began in late October 2018. By early November 2018, bulk earthworks within Stage 1 were generally completed and civil construction (i.e. road construction) began.

In late January 2019, the mucking out of the gully continued south (i.e. vicinity of Lot 300) and the underfill drain and rotten rock mattress was extended here. Subsequently, the construction of retaining walls 2 and 3 commenced (details of the construction of these walls, along with Wall 1 in Stage 2, are provided within Appendix 2 along with the contractor's PS3 and our PS4 certificates).

In early May 2019, the silt pond was decommissioned and the stormwater detention tank in Lot 300 was installed. Retaining wall 2 construction was completed once the tank was installed and bulk filling behind the walls was completed and topsoil was then respreads over all lots.

4 QUALITY ASSURANCE AND CONTROLS

4.1 Site Observations

During the earthworks engineering inspections were undertaken on a regular basis to assess compliance with NZS 4431 and our project specific recommendations and specifications. Project specific inspections were required on this stage of the development for:

 gully areas prior to the placement of fill materials to ascertain that all mullock and soft inorganic subsoils had been removed to our satisfaction;



- installation of underfill drains;
- all aspects of retaining wall construction for walls 1 to 3 (e.g. pile hole excavations, drainage placement etc.).

4.2 Quality Control Criteria

Due to the varying soil types being used as filling, the compaction control criteria of minimum allowable shear strength and maximum allowable air voids were mainly used for quality assurance purposes.

Specification details were as follows:

Minimum Shear Strength and Maximum Air Voids Method

(a) Air Voids Percentage

(As defined in NZS 4402)

General Fill

Average value less than 10% Maximum single value 12%

(b) <u>Undrained Shear Strength</u>

(Measured by Pilcon shear vane - calibrated using NZGS 2001 method)

General fill

Average value not less than 140 kPa
Minimum single value 120 kPa

Note: The average value shall be determined over any ten consecutive tests

In addition, for the placement and compaction of GAP65 hardfill (as was used for the base of the stormwater detention tank area in Lot 300), a minimum Clegg Impact Value (CIV) of 15 was specified.

4.3 Quality Assurance Testing

- (i) In-situ density monitoring was carried out as for the general fill areas and a series of hand auger boreholes were also drilled at selected locations as an added check on quality control.
- (ii) Regular insitu density, strength and water content tests were carried out on all areas of the filling at or in excess of the frequency recommended by NZS 4431.
- (iii) Control tests carried out on the filling showed that on some occasions the required compaction standards were not being achieved.
- (iv)Results of the test failures were relayed to the site foreman and/or his staff, and to the best of our knowledge the affected areas of fill were re-worked as necessary. In each case, further testing was carried out until compliance with the standards was achieved.



5 PROJECT EVALUATION

5.1 Bearing Capacity and Settlement of Building Foundations

Following the completion of earthworks operations, we returned to the site in April 2019 and drilled a series of hand auger boreholes at appropriate natural ground locations in order to determine representative finished ground conditions and hence evaluate likely foundation options for future building development.

At current subgrade levels all filled and undisturbed natural ground has a geotechnical ultimate bearing capacity of 300 kPa within the influence of conventional shallow residential building foundation loads. Where a geotechnical ultimate bearing capacity greater than 300 kPa is required, further specific site investigation and design of foundations should be carried out prior to building consent application.

Where any building platforms have been rutted by heavy machinery, or softened due to ponded rainwater, they should be trimmed back to competent ground and reinstated with compacted hardfill or recompacted clay fill to meet the subdivision's compaction criteria outlined in Section 4.2 to design subgrade level prior to the commencement of building construction.

It should be noted that NZS 3604 only allows a maximum backfill depth of 600mm over the building platform of a dwelling unless an Engineering design solution is proposed, on account of the risk of induced consolidation of the subsoils caused by the weight of the backfill.

5.2 Expansive Soils

Two sets of Expansive soil tests were carried out on samples selected from around the site and within the zone of likely influence of shallow building foundations.

These limit tests were carried out in accordance with NZS 4402, "Methods of Testing Soils for Civil Engineering Purposes" test section 2 and were primarily intended to assess the Expansive Classes of the site materials as defined in AS 2870, "Residential Slabs and Footings – Construction". All test results are IANZ (International Accreditation New Zealand) endorsed and full details are appended.

The AS 2870 Site Class for this subdivision is H1 (high) and a characteristic surface movement (y_s) of up to 60mm can be expected.

We recommend the final building platforms are trimmed and protected immediately using hardfill to minimise desiccation and the potential for post building construction heave beneath the floor slabs / driveways, etc.

5.3 Fill Induced Settlement

As a result of our pre-fill inspections, the installation of subsoil drainage, quality control testing and the elapsed time since the placement of the majority of the filling, we are of the opinion that induced differential settlements beneath or within the certified filling due to its imposed weight should be insignificant with respect to conventional NZS 3604 residential building development.

5.4 Service Trenches

As is normal on all subdivisions, building developments involving foundations within a 45 degree zone of influence from public service pipe inverts will require Engineering input.



5.5 Underfill Drains

The appended cut-fill contours as-built plans show the positions of a perforated underfill drain that was placed in a mucked out gully invert prior to filling to intercept groundwater seepages, as required by NZS 4431. The drain comprises a 160mm-diameter, perforated, heavy duty drain coil embedded in drainage metal and fully wrapped in non-woven geotextile cloth.

The drain was intended to intercept localised groundwater seepages and springs during earthworks and to help provide general control over groundwater levels and were installed as a precautionary measure, not as remedial works for any existing instability, therefore it needs no specific maintenance. This drain presents <u>no</u> foreseeable constraints for future shallow foundations in the aforementioned lots.

Notwithstanding, it is recommended that future foundations or site development works preserve this drain. In the event that it is compromised by any future development of residential lot 21 (albeit unlikely due to the depth of the drain being deeper than 2.0m below existing ground levels within the lots), it should be reinstated under geotechnical engineering observational guidance. Further details are given in Section 6 (3) (f).

5.6 Stormwater Drainage Reserve Lot 300

A stormwater detention tank and overland flow path has been constructed within Lot 300. We were not involved in the design or construction supervision of the detention tank and its outlet structures/conduits.

5.7 Retaining Walls

Some areas of the site have been stabilised by the construction of boundary retaining walls in the locations shown on the finished contours as-built plans (drawing no. AB200). These walls reach a maximum height of approximately 3.5 metres and were designed and inspected by this Consultancy. A copy of our Producer Statement - Construction Review is appended.

An associated Specific Design Zone is shown on the as-built plan set and details of resulting building and earthworks restrictions within the associated Specific Design Zones of these walls are presented in the Suitability Statement.

5.8 Topsoil

Topsoil depths in likely building platform areas were checked by the drilling of a borehole in the approximate centre of each of the residential lots. Our findings, which are indicative only and subject to variation at other locations, show that likely topsoil depths are between 100mm and 200mm.

Site specific findings are presented in the Suitability Statement Summary.

5.9 Contractor's Work

We have relied on the Contractor's work practices and assume that the works have been carried out in accordance with:

- (i) The approved Contract drawings and design details,
- (ii) The approved Contract specifications,



- (iii) Authorised Variations to (i) and (ii) during the execution of the works,
- (iv)The conditions of Resource, Earthworks and Building Consents where applicable,
- (v) The relevant Lander Geotechnical Consultants Limited reports, recommendations and site instructions

and that all as-built information and other details provided to the Client and/or Lander Geotechnical Consultants Limited are accurate and correct in all respects.

6 STATEMENT OF PROFESSIONAL OPINION AS TO THE SUITABILITY OF LAND FOR BUILDING DEVELOPMENT

- I, S.G. Lander, of Lander Geotechnical Consultants Limited, Auckland, hereby confirm that:
- 1. I am a Chartered Professional Engineer experienced in the field of geotechnical engineering as defined in section 1.2.3 of NZS 4404 and was retained by the Owner/Developer as the Geotechnical Engineer on Stage 1 of the 99 Escotts Road, Tuakau residential subdivision.
- 2. The extent of preliminary investigations carried out to date are described in Geotechnical Investigation Report (reference J00779 dated 28 March 2018) and Geotechnical Design Report (reference J00779, dated 9 May 2018) and the conclusions and recommendations of those documents have been re-evaluated in the preparation of this report. The results of all tests carried out are appended.
- 3. In my professional opinion, not to be construed as a guarantee, I consider that:
 - (a) The earth fills shown on the appended fill as-built plan have been placed in compliance with / NZS 4431 and related documents.
 - (b) The completed earthworks give due regard to land slope and foundation stability considerations within residential lots.
 - (c) A geotechnical ultimate bearing capacity of 300 kPa may be assumed for foundation design on all residential lots. Where a geotechnical bearing capacity greater than 300 kPa is required, (ie. outside the limits of NZS 3604, such as when piling is undertaken), further specific site investigation and design of foundations should be carried out prior to building consent application.
 - (d) The backfilling and compaction of the live stormwater and sanitary sewer trenches on this subdivision has where possible been carried out to appropriate standards having regard for the prevailing ground conditions and associated compaction induced pipe loadings.
 - Nevertheless, no building development should take place within the 45 degree zone of influence of drain inverts unless endorsed by specific site investigations, foundation designs and by construction inspections undertaken by a Chartered Professional Engineer experienced in geomechanics to ensure that lateral stability and differential settlement issues are addressed and that building loads are transferred beyond the influence of the pipe and beyond the extent of the trench backfill.
 - (e) No building construction, including the construction of additional retaining walls and no earthworks should take place should take place within a horizontal distance equal to the wall face height (measured from the top of the wall) behind the cantilever pole retaining walls on lots 58 to



- 61 (as hatched by the Specific Design Zone on the finished contours plans) unless endorsed by specific designs and by construction inspections undertaken by a Chartered Professional Engineer experienced in geomechanics to ensure that no additional loads are applied to the walls. Specific site investigation should not be required.
- (f) The function of the underfill drain which passes through lot 21 must not be impaired by any building development or landscaping works. In particular, any bored or driven piles must be positioned to avoid damaging this drain. The drain is at a minimum depth of 2m below existing ground level and should therefore not pose any risk to shallow building foundations constructed in accordance with the recommendations of this report.
- (g) The assessed AS 2870 expansive site Class for all lots is H1 (high) and the characteristic ground surface movement (y_s) is up to 60mm.
- (h) Subject to the geotechnical limitations, restrictions, recommendations and expansive soil assessments associated with 3(b), 3(c), 3(d), 3(e), 3(f), and 3(g) above:
 - (i) The filled and undisturbed original ground within residential lot boundaries is generally suitable for residential buildings constructed in accordance with NZS 3604 and related documents.
 - (ii) On all lots, foundation design may be carried out in accordance with AS 2870 (Class H1) or alternatively, a specific foundation and structural design may be undertaken by a Chartered Professional Engineer who should allow for expansive soil effects in the design. In this latter case, the minimum recommended foundation depth below cleared ground level following topsoil removal and benching of building platform areas is 900mm.
 - For buildings having brittle exterior cladding appropriate control joints should also be specifically designed depending on architectural specifications and structural form.
- 4. Road subgrades (i.e. Roads 1 to 4) within Stage 1 have been formed having due regard for slope stability and settlement, although available subgrade strengths are dependent on site conditions and variable results should be expected.
- 5. The Stormwater Drainage Reserve (Lot 300) has been formed to a standard suitable for its proposed end use.

The professional opinion contained within this report is furnished to the Waikato District Council and Hughes Developments Limited for their purposes alone on the express condition that it will not be relied upon by any other person. Prospective purchasers should still satisfy themselves as to any specific conditions pertaining to their particular land interest.

The appended table summarises the status of each residential lot covered by this Suitability Statement.



For and on behalf of Lander Geotechnical Consultants Limited

Prepared By:

Michael Chan

Geotechnical Project Engineer NZDE(Civil)

Authorised By:

Shane Lander

Principal Geotechnical Engineer CMEngNZ, CPEng., IntPE(NZ)

Reviewed and Authorised By:

Chris Edwards

Senior Engineering Geologist CMEngNZ(PEngGeol)



Table 2: Suitability Statement Summary

Lot No.	Comments	Topsoil Depth (mm)	Ultimate Bearing (kPa)	AS2870 :2011 Class
1	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	100	300	H1
2	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	100	300	H1
3	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	100	300	H1
4	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	100	300	H1
5	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	150	300	H1
6	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	150	300	H1
7	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	200	300	H1
8	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	100	300	H1
9	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	100	300	H1
10	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	150	300	H1
11	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	150	300	H1
12	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	150	300	H1
13	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	150	300	H1
14	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	150	300	H1
15	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	100	300	H1
16	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	150	300	H1
17	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	100	300	H1
18	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	100	300	H1
19	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	100	300	H1



Lot No.	Comments	Topsoil Depth (mm)	Ultimate Bearing (kPa)	AS2870 :2011 Class
20	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	100	300	H1
21	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm. Function of the underfill drain must be preserved (refer Section 6 (3) (f) of report)	100	300	H1
22	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	100	300	H1
23	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	100	300	H1
24	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	150	300	H1
25	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	100	300	H1
26	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	150	300	H1
27	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	150	300	H1
28	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	150	300	H1
29	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	200	300	H1
30	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	100	300	H1
31	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	100	300	H1
32	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	150	300	H1
33	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	150	300	H1
34	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	100	300	H1
35	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	150	300	H1
36	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	100	300	H1
37	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	100	300	H1
38	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	100	300	H1



Lot No.	Comments	Topsoil Depth (mm)	Ultimate Bearing (kPa)	AS2870 :2011 Class
39	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	100	300	H1
40	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	100	300	H1
41	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	100	300	H1
42	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	100	300	H1
55	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	150	300	H1
56	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	150	300	H1
57	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	200	300	H1
58	No construction or earthworks within specific design zone unless endorsed by specific foundation designs and construction inspections due to presence of cantilever pole retaining wall. Elsewhere, AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	200	300	H1
59	No construction or earthworks within specific design zone unless endorsed by specific foundation designs and construction inspections due to presence of cantilever pole retaining wall. Elsewhere, AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	150	300	Н1
60	No construction or earthworks within specific design zone unless endorsed by specific foundation designs and construction inspections due to presence of cantilever pole retaining wall. Elsewhere, AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	150	300	H1
61	No construction or earthworks within specific design zone unless endorsed by specific foundation designs and construction inspections due to presence of cantilever pole retaining wall. Elsewhere, AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	150	300	H1
62	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	100	300	H1
63	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	200	300	H1



Lot No.	Comments	Topsoil Depth (mm)	Ultimate Bearing (kPa)	AS2870 :2011 Class
64	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	200	300	H1
65	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	150	300	H1
66	AS 2870 foundation design or NZS 3604 with minimum footing depth 900mm.	100	300	H1

Appendix 1

CivilPlan Consultants Limited As-Built Drawings



HUGHES DEVELOPMENTS LIMITED 99 ESCOTTS ROAD TUAKAU

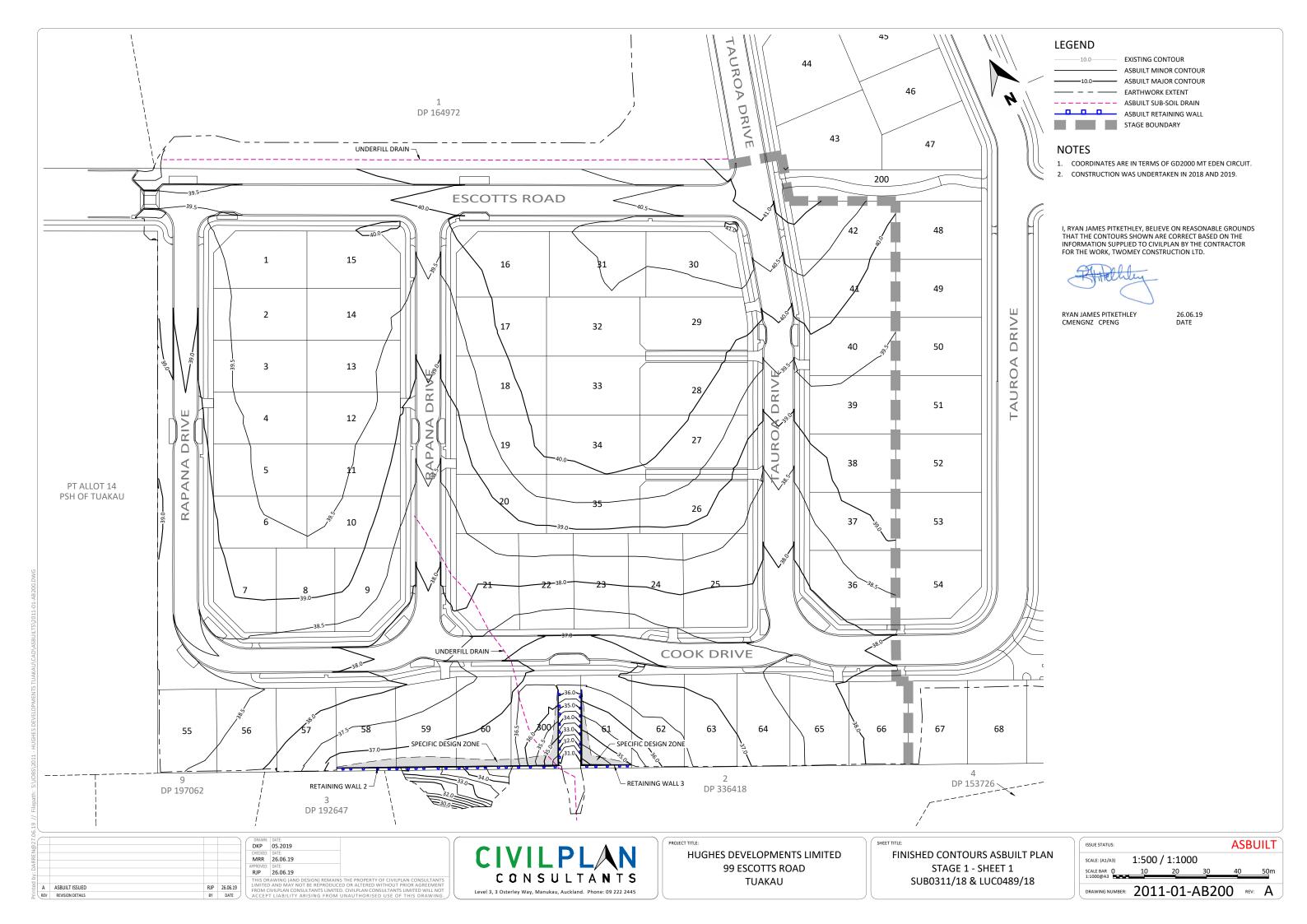
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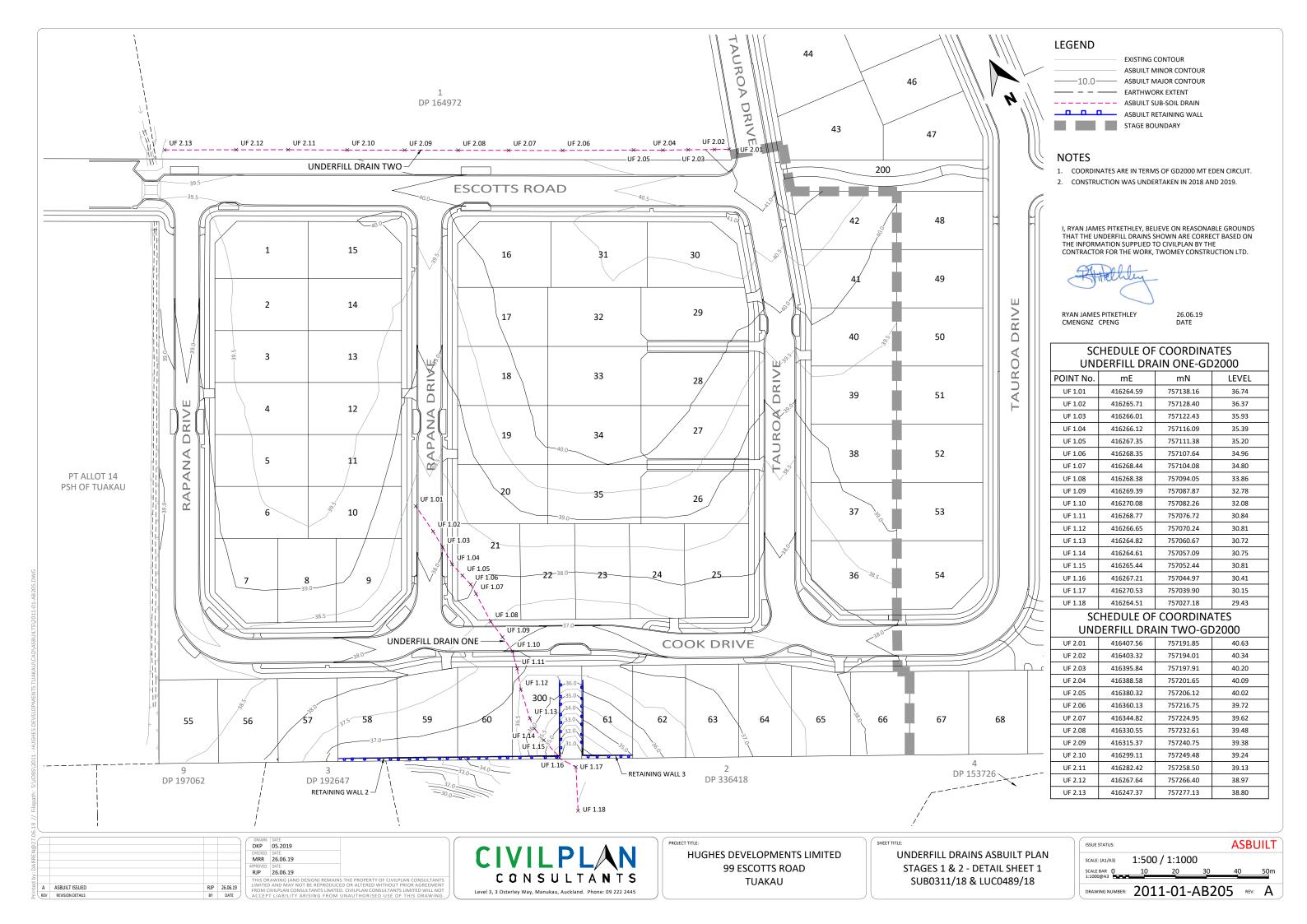
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Sheet Number Sheet Title				
2011-01-AB000	COVER SHEET			
200 Earthworks				
2011-01-AB200	EARTHWORKS ASBUILT-FINISHED CONTOURS			
2011-01-AB205	UNDERFILL DRAIN ASBUILT PLAN-OVERALL LAYOUT			
2011-01-AB220	ISOPACH ASBUILT PLAN-EXISTING TO FINISHED SURFACE			
2011-01-AB225	ISOPACH ASBUILT PLAN-STRIPPED TO SUBGRADE SURFACE			
2011-01-AB270	RETAINING WALL ASBUILT PLAN-OVERALL LAYOUT			
2011-01-AB271	RETAINING WALL ASBUILT PLAN-DETAIL SHEET			
2011-01-AB290	SLOPE ANALYSIS ASBUILT PLAN			
300 Roading				
2011-01-AB300	ROADING ASBUILT-PAVEMENT PLAN			
2011-01-AB301	ROADING ASBUILT-FEATURES PLAN			
400 Drainage				
2011-01-AB400	STORMWATER ASBUILT-OVERALL LAYOUT			
2011-01-AB401	STORMWATER ASBUILT-DETAIL SHEET 1			
2011-01-AB402	STORMWATER ASBUILT-DETAIL SHEET 2			
2011-01-AB403	STORMWATER ASBUILT-DETAIL SHEET 3			
2011-01-AB404	STORMWATER ASBUILT-DETAIL SHEET 4			
2011-01-AB405	STORMWATER ASBUILT-RAIN GARDEN DETAIL SHEET 1			
2011-01-AB406	STORMWATER ASBUILT-RAIN GARDEN DETAIL SHEET 2			
2011-01-AB407	STORMWATER ASBUILT-RAIN GARDEN DETAIL SHEET 3 & COORDINATES			
2011-01-AB410	WASTEWATER ASBUILT-OVERALL LAYOUT			
2011-01-AB411	WASTEWATER ASBUILT-DETAIL SHEET 1			
2011-01-AB412	WASTEWATER ASBUILT-DETAIL SHEET 2			
2011-01-AB413	WASTEWATER ASBUILT-DETAIL SHEET 3			
2011-01-AB414	WASTEWATER ASBUILT-DETAIL SHEET 4			
2011-01-AB415	WASTEWATER ASBUILT-DETAIL SHEET 5			
2011-01-AB416	WASTEWATER ASBUILT-DETAIL SHEET 6			

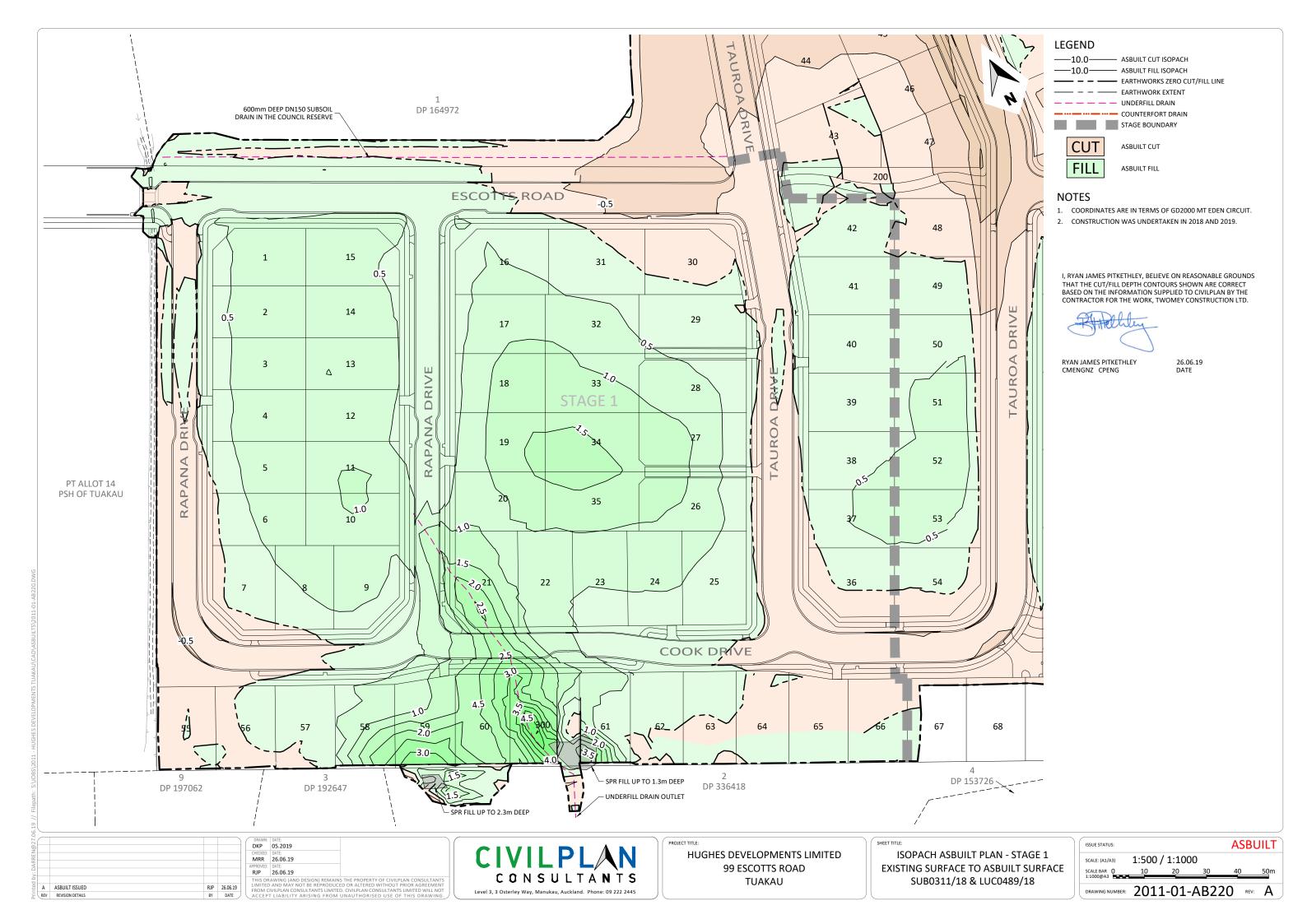
500 Water Supply	
2011-01-AB500	WATER SUPPLY ASBUILT-OVERALL LAYOUT-SHEET 1
2011-01-AB501	WATER SUPPLY ASBUILT-OVERALL LAYOUT-SHEET 2
2011-01-AB502	WATER SUPPLY ASBUILT-DETAIL SHEET 1
2011-01-AB503	WATER SUPPLY ASBUILT-DETAIL SHEET 2
2011-01-AB504	WATER SUPPLY ASBUILT-DETAIL SHEET 3
2011-01-AB505	WATER SUPPLY ASBUILT-DETAIL SHEET 4
2011-01-AB506	WATER SUPPLY ASBUILT-COORDINATE SCHEDULES
600 Services	
2011-01-AB600	POWER SUPPLY ASBUILT-STAGE 1-SHEET 1
2011-01-AB601	POWER SUPPLY ASBUILT-STAGE 1-SHEET 2
2011-01-AB610	CHORUS ASBUILT PLAN-STAGE 1-SHEET 1

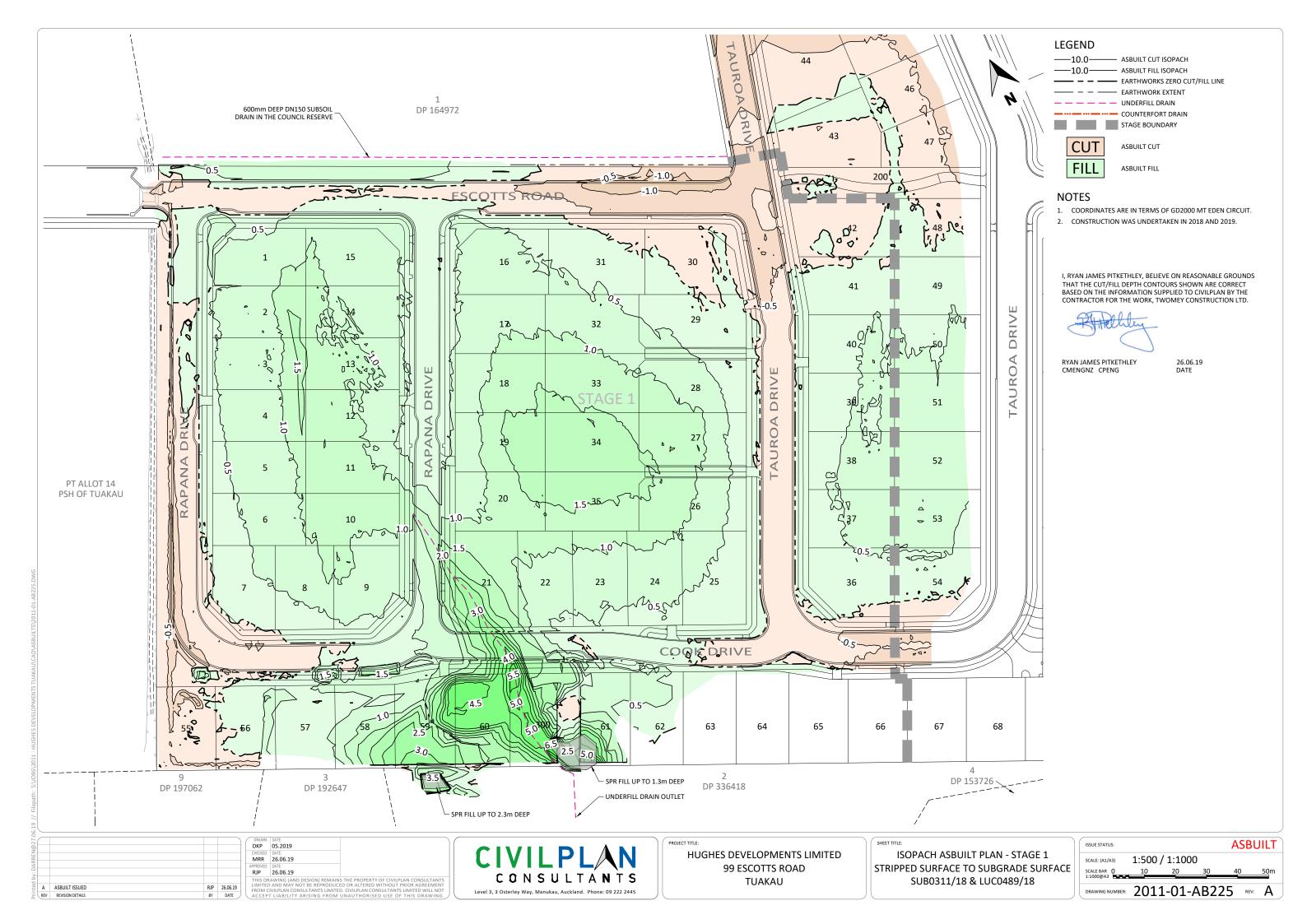
AS-BUILT ISSUE JUNE 2019

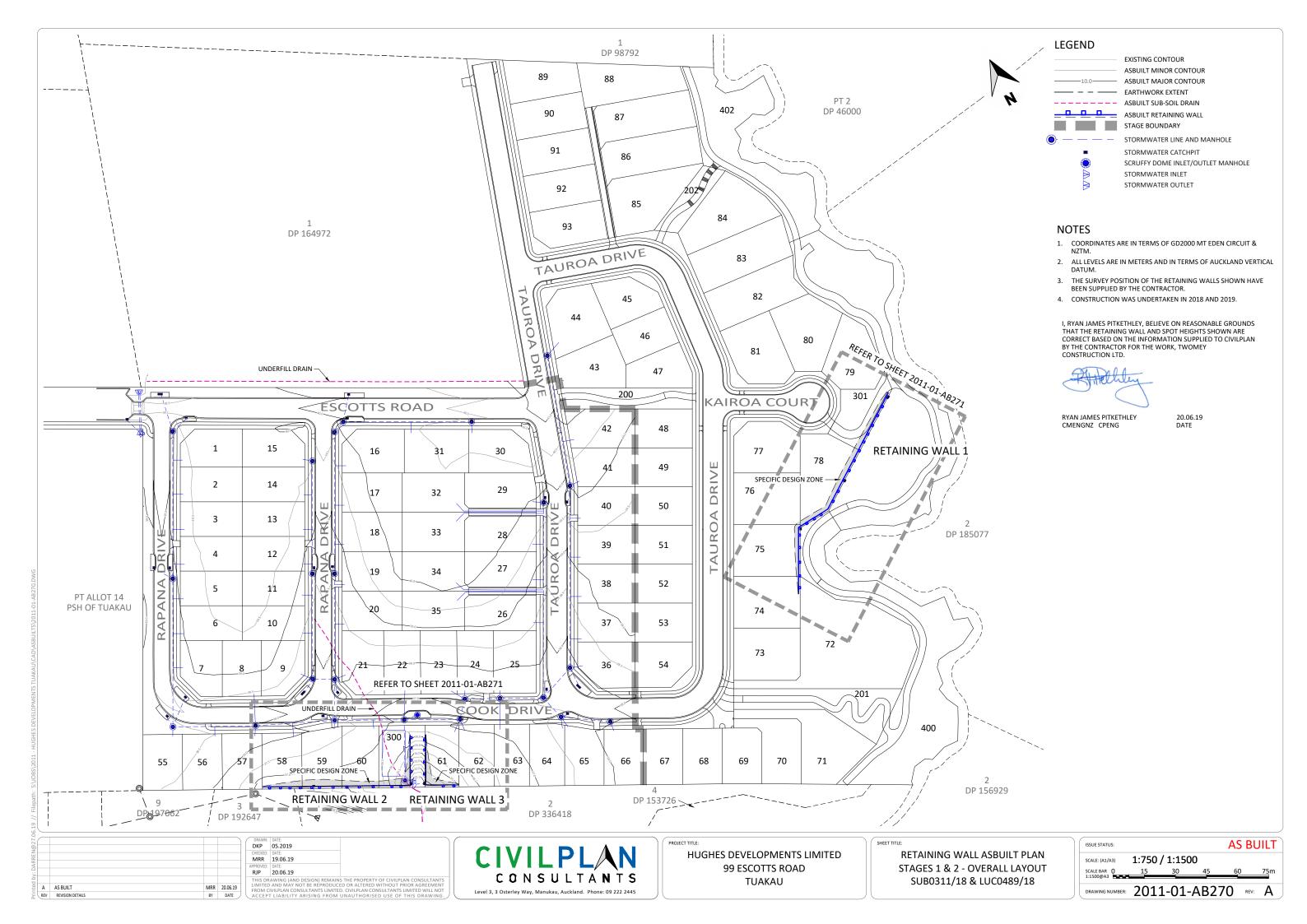


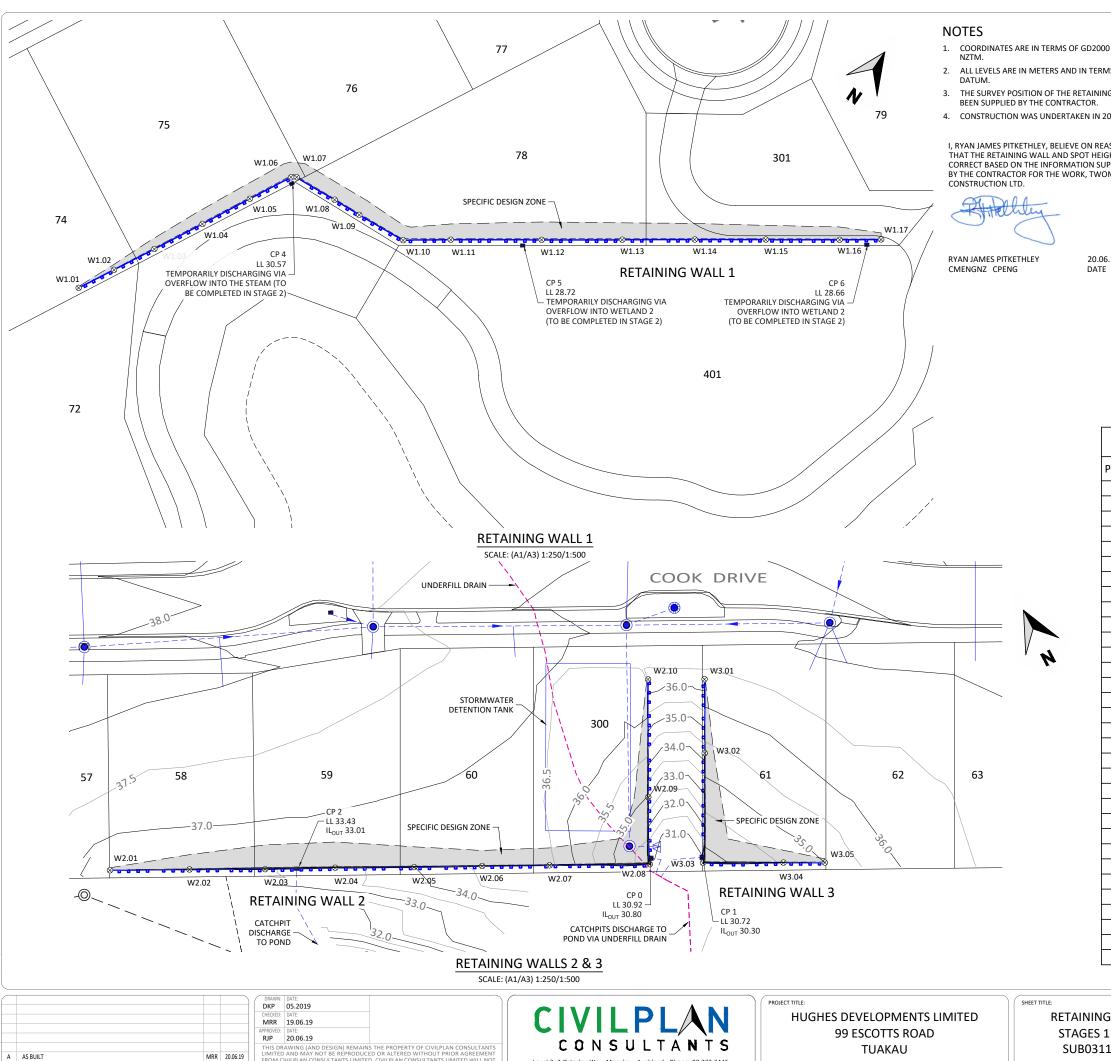












REV REVISION DETAIL

LEGEND 1. COORDINATES ARE IN TERMS OF GD2000 MT EDEN CIRCUIT & NZTM. **EXISTING CONTOUR** ASBUILT MINOR CONTOUR ALL LEVELS ARE IN METERS AND IN TERMS OF AUCKLAND VERTICAL DATUM. ASBUILT MAJOR CONTOUR

3. THE SURVEY POSITION OF THE RETAINING WALLS SHOWN HAVE BEEN SUPPLIED BY THE CONTRACTOR.

4. CONSTRUCTION WAS UNDERTAKEN IN 2018 AND 2019.

I, RYAN JAMES PITKETHLEY, BELIEVE ON REASONABLE GROUNDS THAT THE RETAINING WALL AND SPOT HEIGHTS SHOWN ARE CORRECT BASED ON THE INFORMATION SUPPLIED TO CIVILPLAN BY THE CONTRACTOR FOR THE WORK, TWOMEY

20.06.19

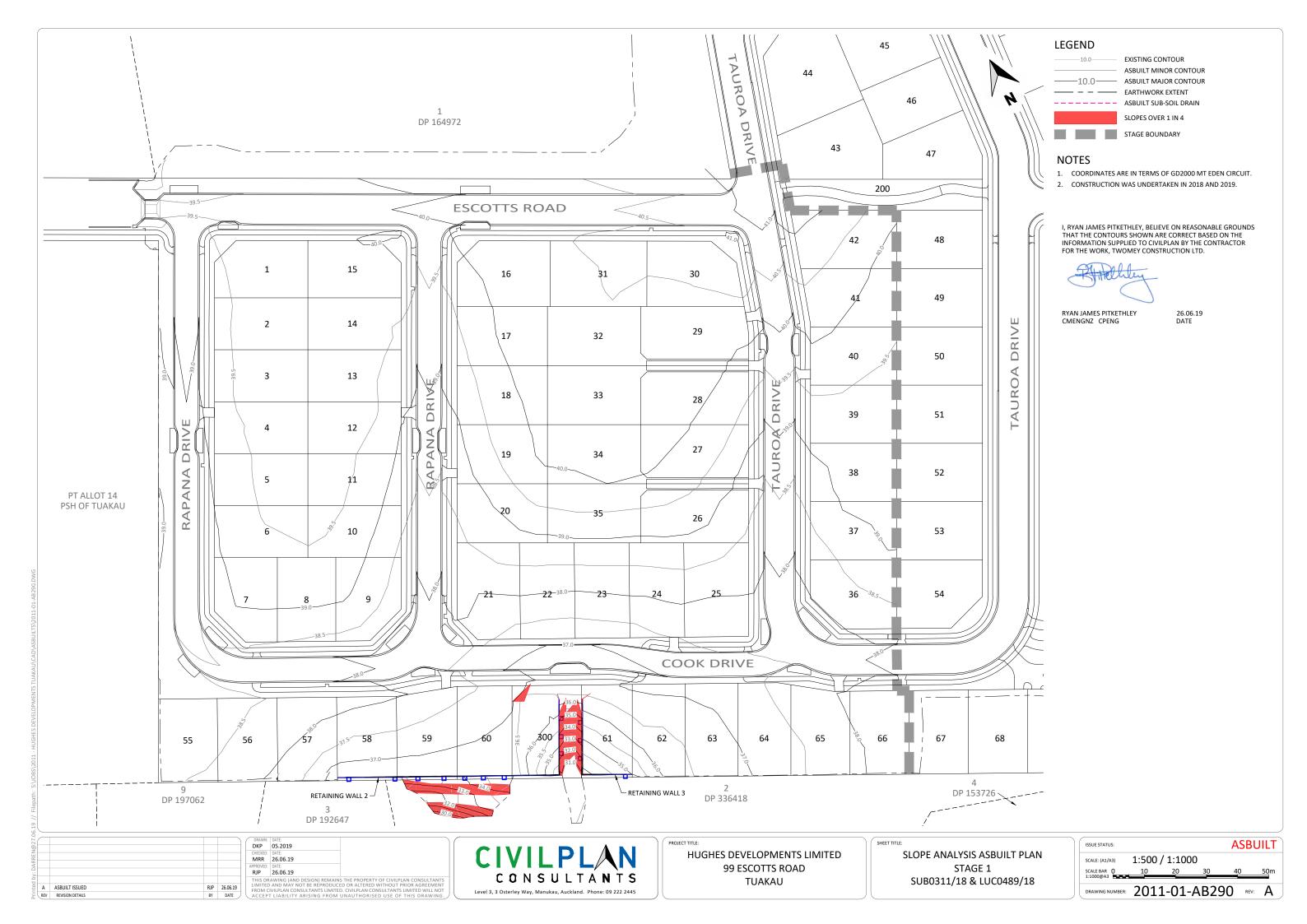
EARTHWORK EXTENT ---- ASBUILT SUB-SOIL DRAIN ASBUILT RETAINING WALL STAGE BOUNDARY STORMWATER LINE AND MANHOLE STORMWATER CATCHPIT SCRUFFY DOME INLET/OUTLET MANHOLE STORMWATER INLET STORMWATER OUTLET

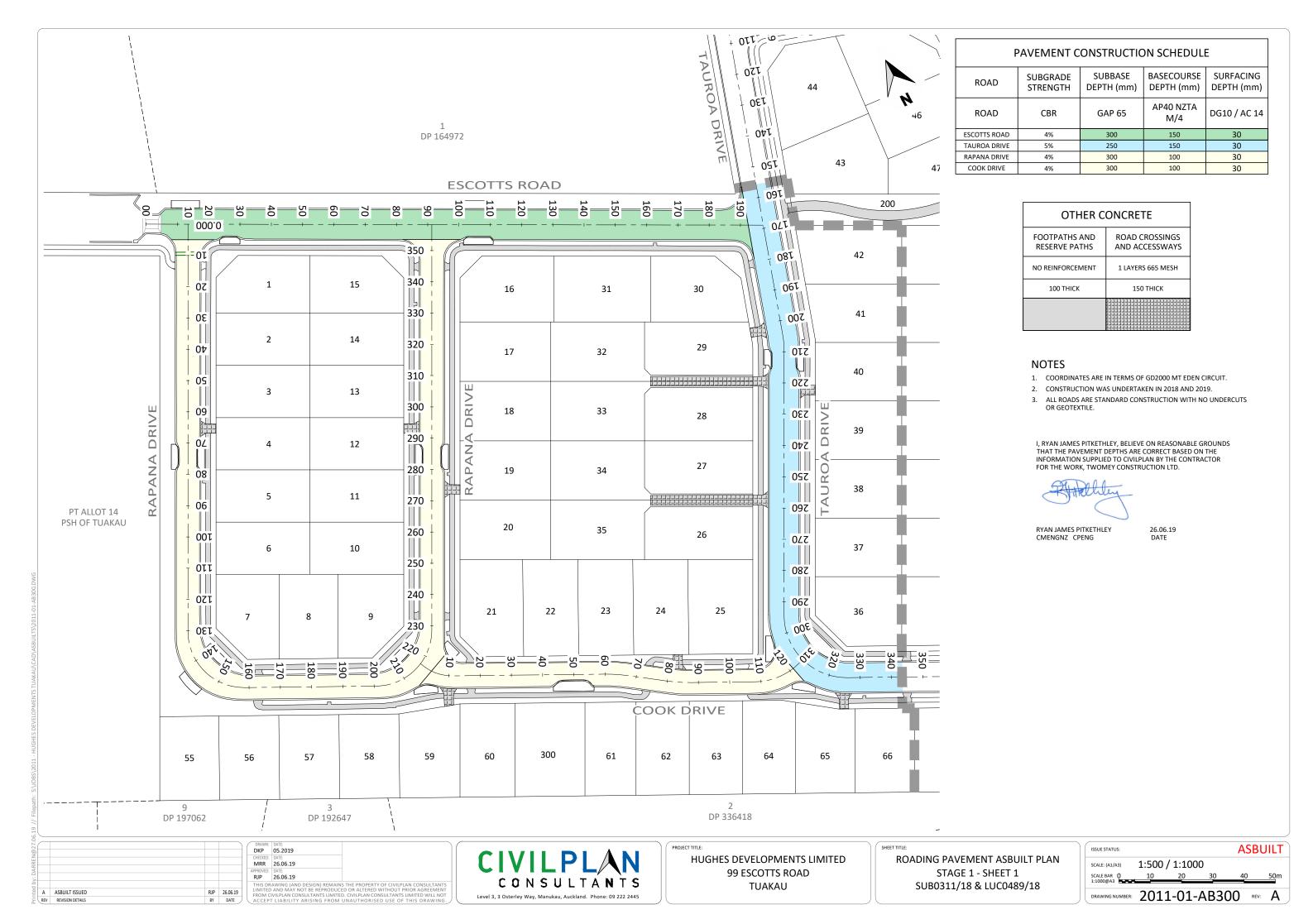
SCHEDULE OF COORDINATES						
RETAINING WALLS 1-3-GD2000						
POINT No.	mE	mN	LEVEL BOTTOM	LEVEL TOP	HEIGHT (m)	
W1.01	416476.23	757038.70	32.43	32.59	0.16	
W1.02	416478.74	757043.31	31.94	32.54	0.60	
W1.03	416481.59	757048.59	31.24	32.50	1.26	
W1.04	416484.95	757054.93	30.80	32.48	1.68	
W1.05	416488.24	757061.22	30.61	32.47	1.86	
W1.06	416491.15	757066.64	30.47	32.49	2.02	
W1.07	416491.77	757067.07	30.38	32.49	2.11	
W1.08	416497.58	757067.45	29.64	31.73	2.09	
W1.09	416501.40	757067.65	29.13	31.27	2.14	
W1.10	416508.09	757068.15	28.49	30.51	2.02	
W1.11	416513.25	757071.77	28.62	30.71	2.09	
W1.12	416523.16	757078.54	28.73	31.10	2.37	
W1.13	416531.96	757084.58	29.21	31.46	2.25	
W1.14	416539.85	757090.02	29.59	31.73	2.14	
W1.15	416547.61	757095.32	28.98	30.87	1.89	
W1.16	416555.68	757100.82	28.29	30.00	1.71	
W1.17	416560.19	757103.96	28.65	29.52	0.87	
W2.01	416204.39	757078.34	36.35	36.69	0.34	
W2.02	416213.72	757073.48	34.69	36.64	1.95	
W2.03	416222.56	757068.80	33.55	36.68	3.13	
W2.04	416230.84	757064.59	33.27	36.68	3.41	
W2.05	416240.07	757059.76	33.99	36.65	2.66	
W2.06	416248.08	757055.66	34.61	36.68	2.07	
W2.07	416255.99	757051.54	34.33	36.64	2.31	
W2.08	416267.75	757045.43	30.70	34.18	3.48	
W2.09	416271.74	757053.33	33.04	34.99	1.95	
W2.10	416279.06	757067.09	36.37	36.51	0.14	
W3.01	416285.65	757063.58	36.29	36.40	0.11	
W3.02	416281.08	757054.94	34.07	35.34	1.27	
W3.03	416274.05	757042.27	30.66	33.81	3.15	
W3.04	416283.43	757037.28	32.91	34.57	1.66	
W3.05	416288.29	757034.70	34.52	35.01	0.49	

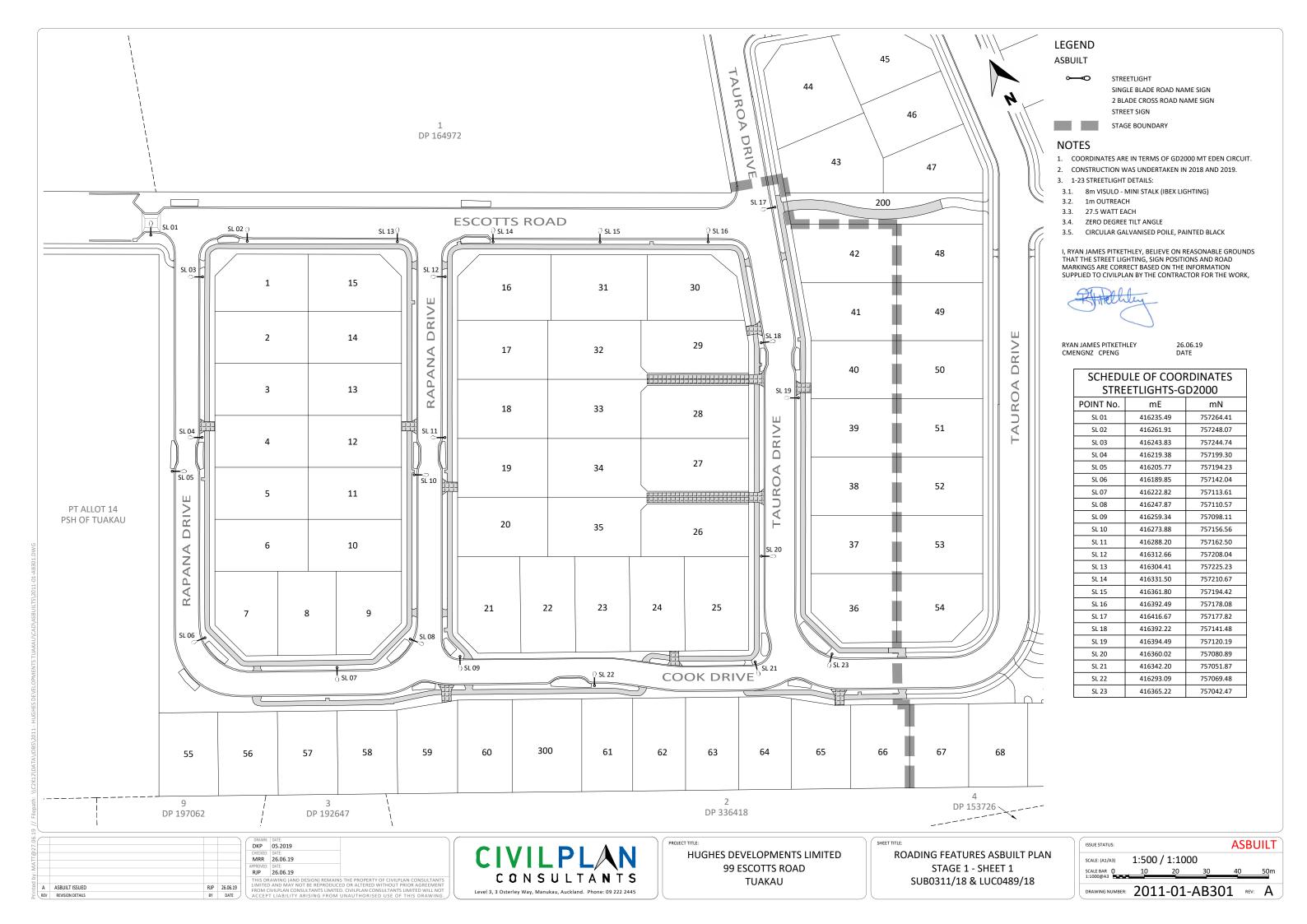
COLLEGE OF COORDINATES

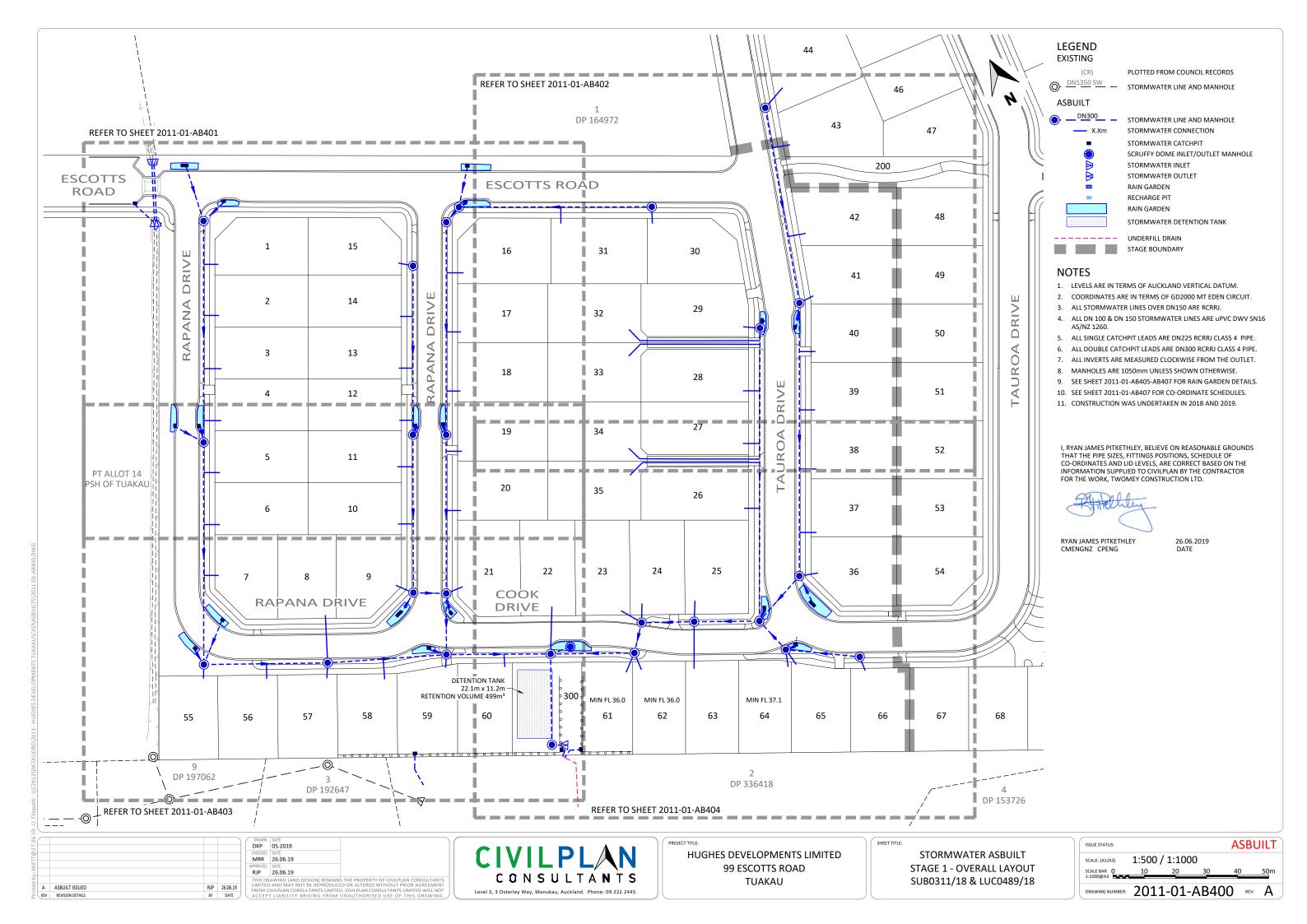
RETAINING WALL ASBUILT PLAN STAGES 1 & 2 - DETAIL SHEET SUB0311/18 & LUC0489/18

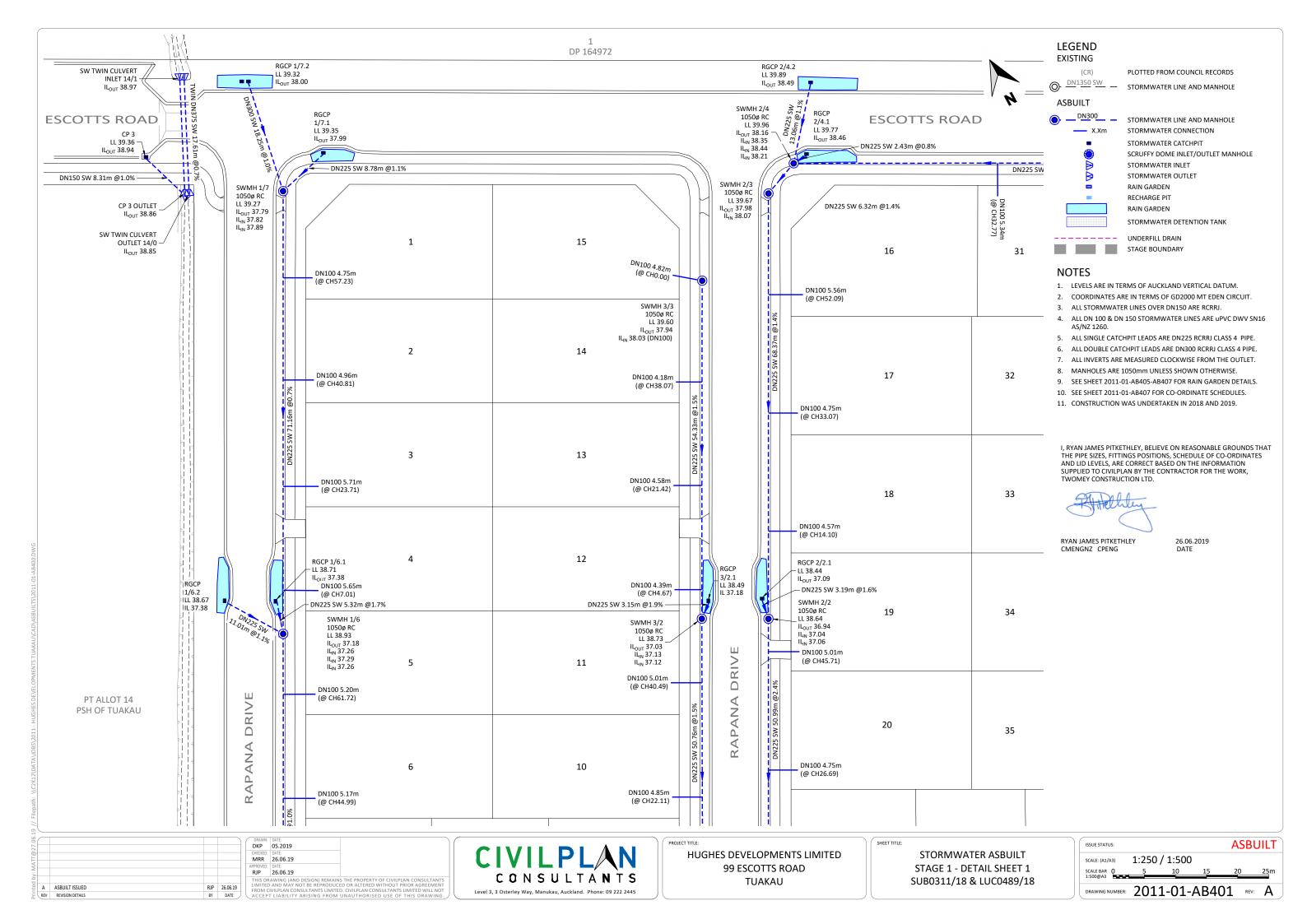
ISSUE STATUS: **AS BUILT** 1:250 / 1:500 DRAWING NUMBER: 2011-01-AB271 REV: A

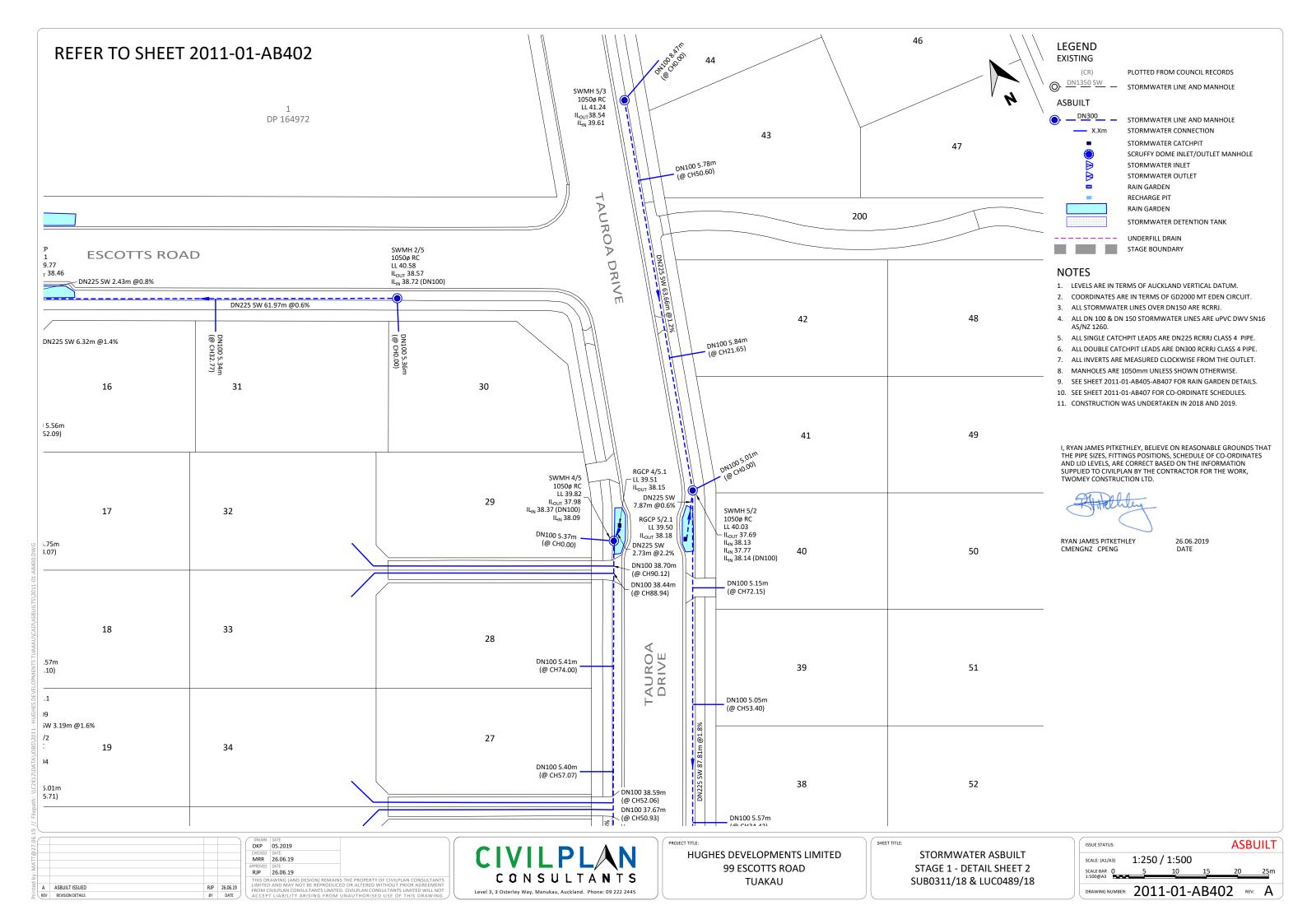


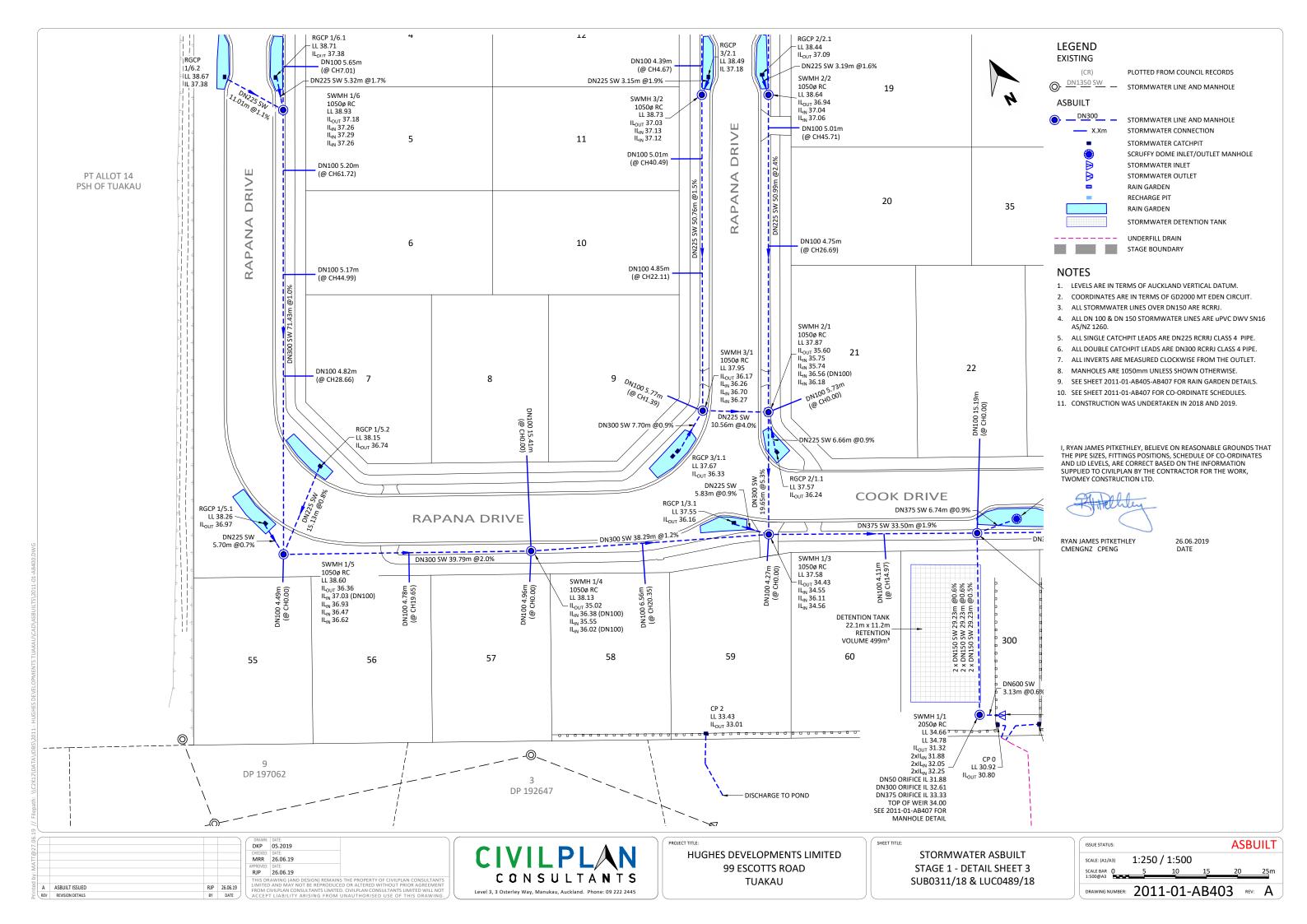


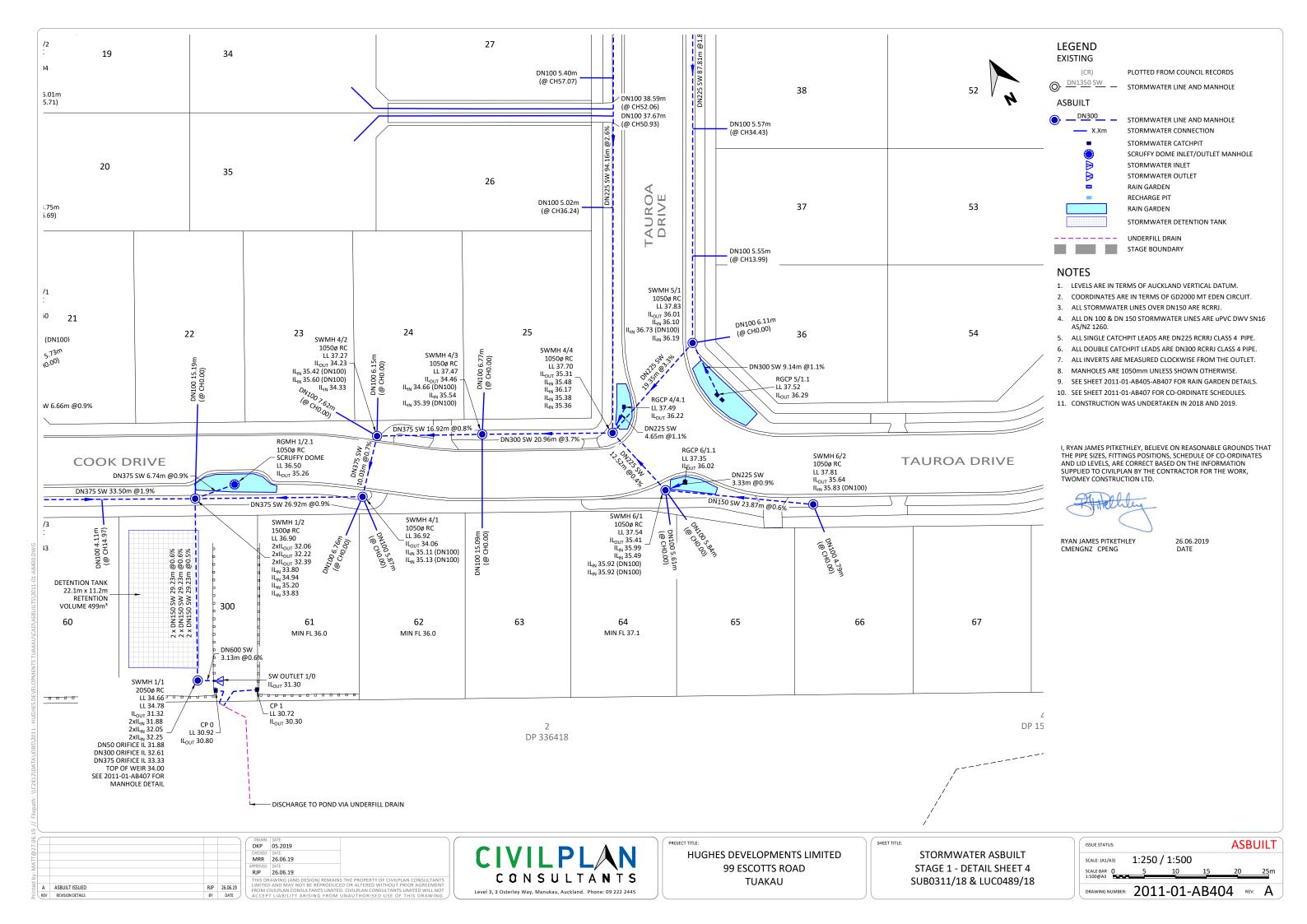


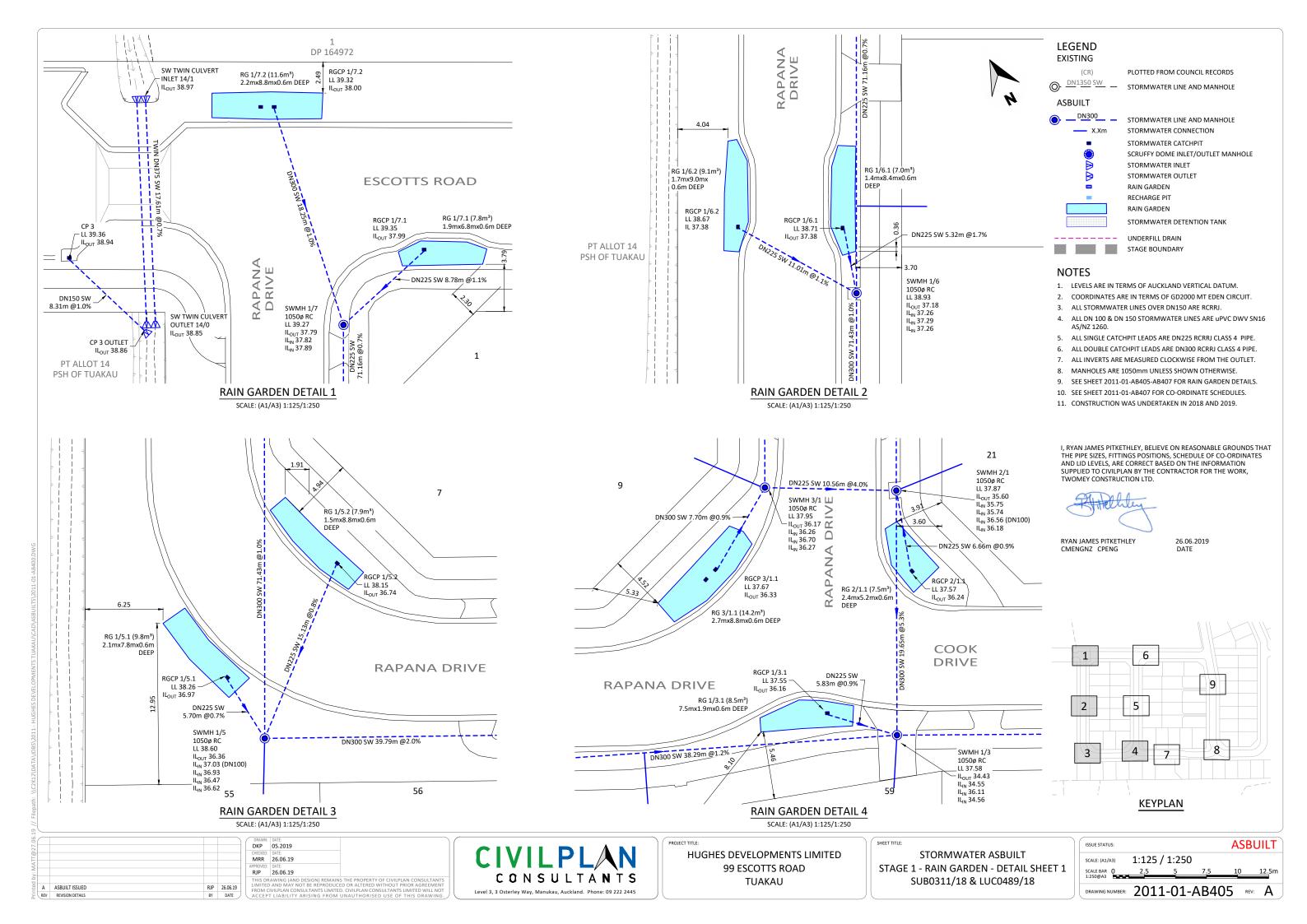


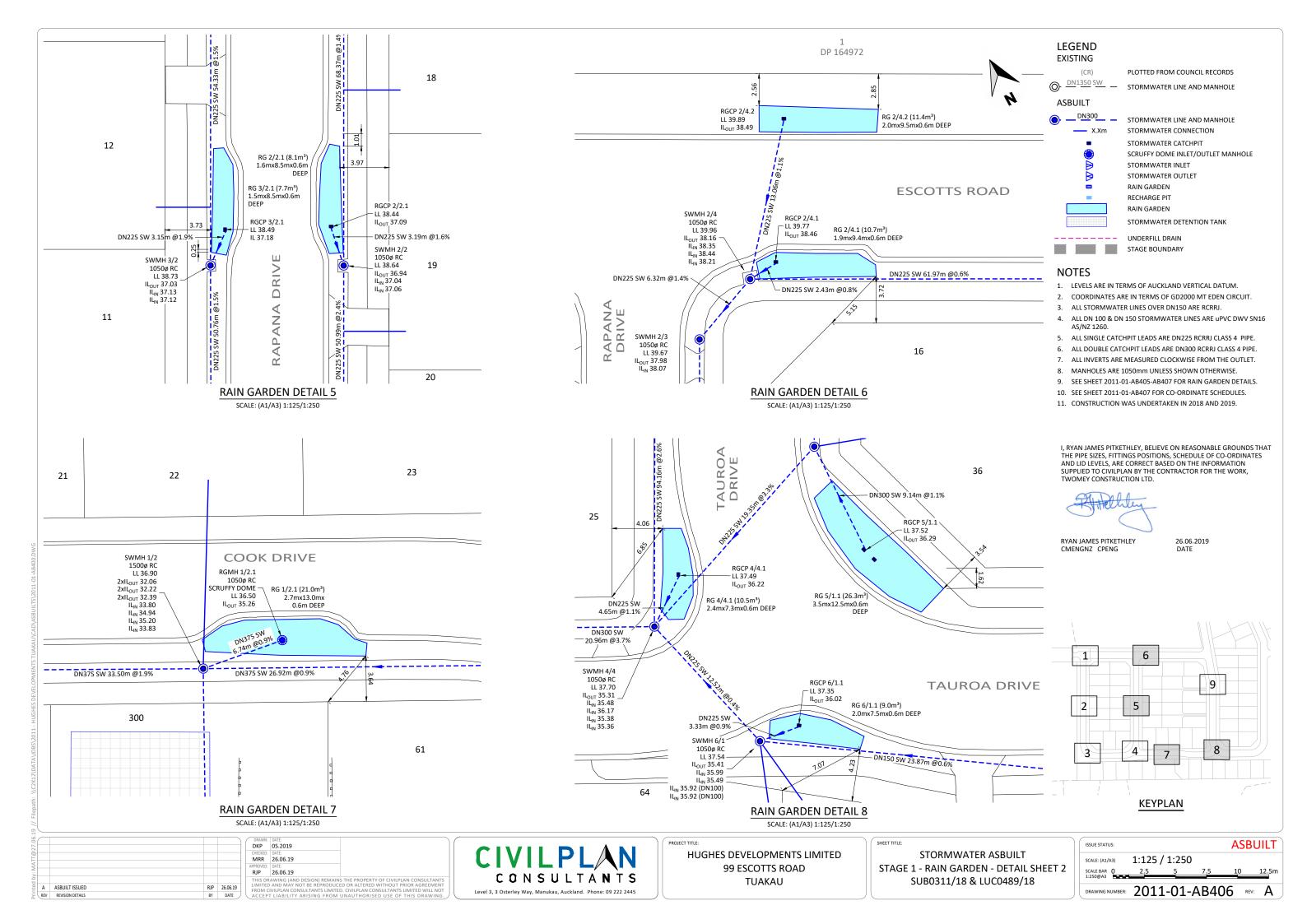


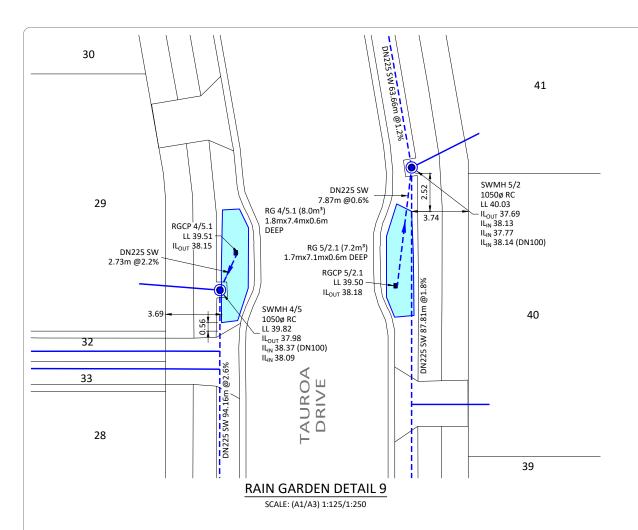


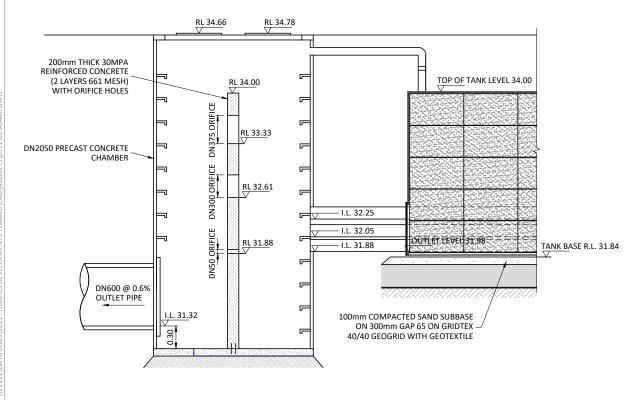












SCHEDULE OF COORDINATES SW MANHOLES-GD2000 POINT No. SWMH 1/0 416269.23 757047.26 SWMH 1/1 416266.50 757048.79 SWMH 1/2 416279.91 757074.76 SWMH 1/3 416250.26 757090.35 SWMH 1/4 416215.31 757105.98 SWMH 1/5 416179.97 757124.26 SWMH 1/6 SWMH 1/7 757250.07 SWMH 2/1 416259.47 757107.71 SWMH 2/2 757152.69 SWMH 2/3 416315.70 757212.99 SWMH 2/4 416321.57 757215.35 SWMH 2/5 416376.27 757186.22 SWMH 3/1 416250.27 757112.89 SWMH 3/2 416274.06 757157.74 SWMH 3/3 416299.70 757205.63 416303.80 SWMH 4/1 757062.35 SWMH 4/2 416310.45 757069.87 SWMH 4/3 416325.50 757062 14 416344.09 SWMH 4/4 757052 47 416388.61 757135 44 SWMH 4/5 416362.23 757059 17 SWMH 5/1 SWMH 5/2 416403.61 757136.62 SWMH 5/3 416423.48 757197.10 SWMH 6/1 416347.23 757040.35

SCHEDULE OF COORDINATES					
RAINGARDEN CATCHPITS-GD2000					
POINT No.	mE	mN			
RG 1/2.1	416286.58	757073.79			
RG 1/3.1	416246.12	757094.44			
RG 1/5.1	416179.53	757129.94			
RG 1/5.2	416191.64	757133.89			
RG 1/6.1	416215.14	757192.38			
RG 1/6.2	416207.56	757196.53			
RG 1/7.1	416255.58	757252.25			
RG 1/7.2	416250.35	757268.04			
RG 2/1.1	416257.43	757101.37			
RG 2/2.1	416284.02	757155.83			
RG 2/4.1	416323.99	757215.49			
RG 2/4.2	416329.98	757225.34			
RG 3/1.1	416243.72	757108.85			
RG 3/2.1	416276.54	757159.67			
RG 4/4.1	416347.86	757055.20			
RG 4/5.1	416390.80	757137.07			
RG 5/1.1	416361.97	757050.03			
RG 5/2.1	416399.09	757130.17			
RG 6/1.1	416350.53	757039.90			

416367.11

757027.14

SWMH 6/2

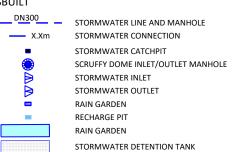
SCHEDULE OF COORDINATES CATCHPITS-GD2000					
POINT No. mE mN					
CP 0	416268.13	757046.12			
CP 1	416274.42	757042.89			
CP 2 416226.18 757066.69					



LEGEND EXISTING

(CR) PLOTTED FROM COUNCIL RECORDS $\bigcirc \hspace{-0.5cm} \stackrel{DN1350}{\odot} \stackrel{SW}{-} \hspace{-0.5cm} - \hspace{-0.5cm} \text{STORMWATER LINE AND MANHOLE}$

ASBUILT



---- UNDERFILL DRAIN STAGE BOUNDARY

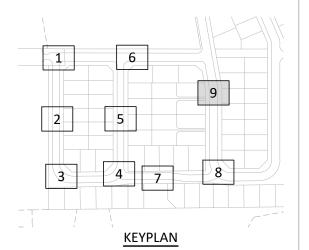
NOTES

- 1. LEVELS ARE IN TERMS OF AUCKLAND VERTICAL DATUM.
- 2. COORDINATES ARE IN TERMS OF GD2000 MT EDEN CIRCUIT.
- . ALL STORMWATER LINES OVER DN150 ARE RCRIJ.
- ALL DN 100 & DN 150 STORMWATER LINES ARE uPVC DWV SN16 AS/N7 1260
- 5. ALL SINGLE CATCHPIT LEADS ARE DN225 RCRRJ CLASS 4 PIPE.
- 6. ALL DOUBLE CATCHPIT LEADS ARE DN300 RCRRJ CLASS 4 PIPE.
- 7. ALL INVERTS ARE MEASURED CLOCKWISE FROM THE OUTLET.
- MANHOLES ARE 1050mm UNLESS SHOWN OTHERWISE.
- 9. SEE SHEET 2011-01-AB405-AB407 FOR RAIN GARDEN DETAILS.
- 10. SEE SHEET 2011-01-AB407 FOR CO-ORDINATE SCHEDULES.
- 11. CONSTRUCTION WAS UNDERTAKEN IN 2018 AND 2019.

I, RYAN JAMES PITKETHLEY, BELIEVE ON REASONABLE GROUNDS THAT THE PIPE SIZES, FITTINGS POSITIONS, SCHEDULE OF CO-ORDINATES AND LID LEVELS, ARE CORRECT BASED ON THE INFORMATION SUPPLIED TO CIVILPLAN BY THE CONTRACTOR FOR THE WORK, TWOMEY CONSTRUCTION LTD.

RYAN JAMES PITKETHLEY CMENGNZ CPENG

26.06.2019



STORMWATER MANHOLE 1/1 DETAIL SCALE: (A1/A3) 1:125/1:250

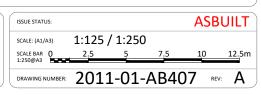


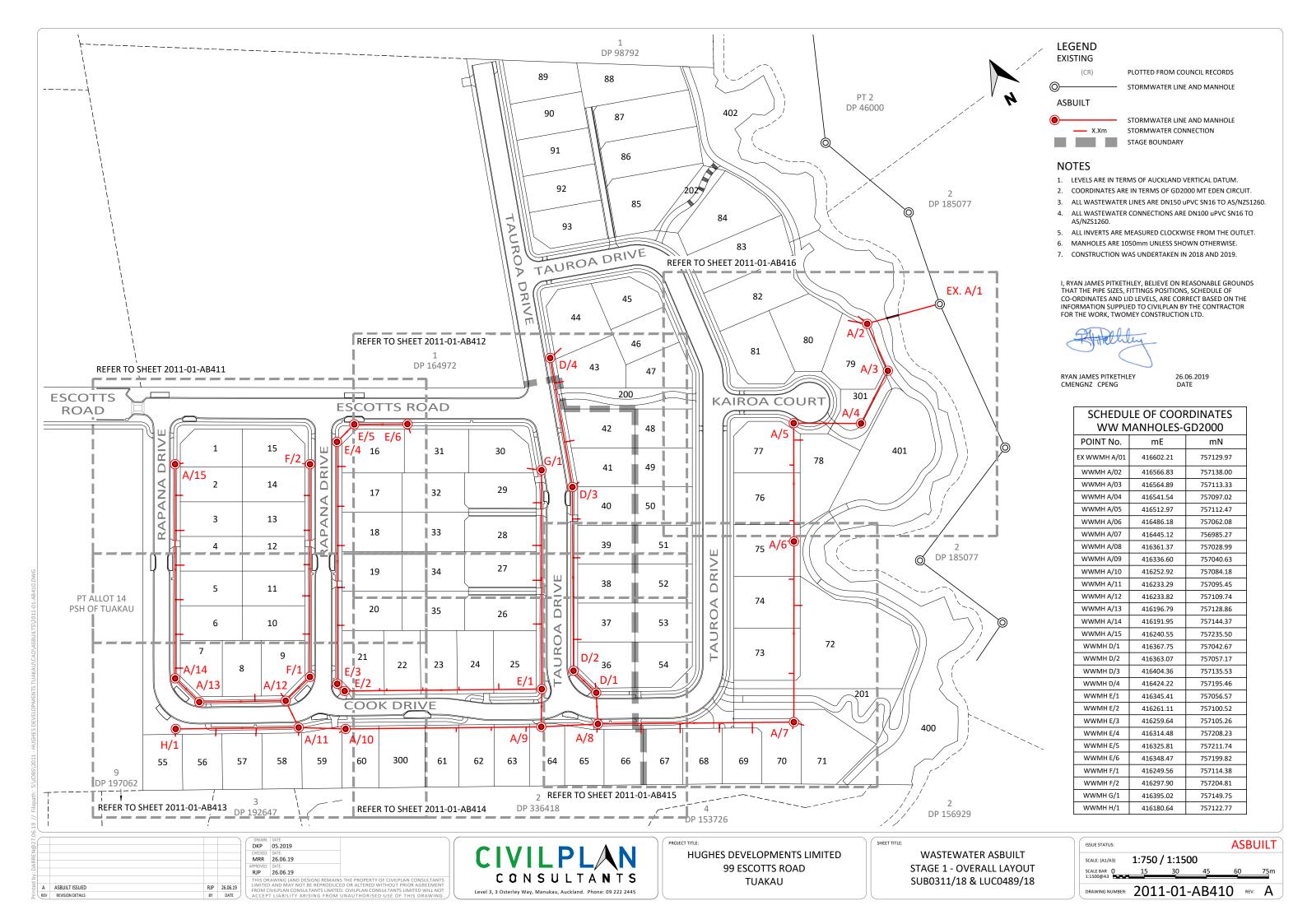


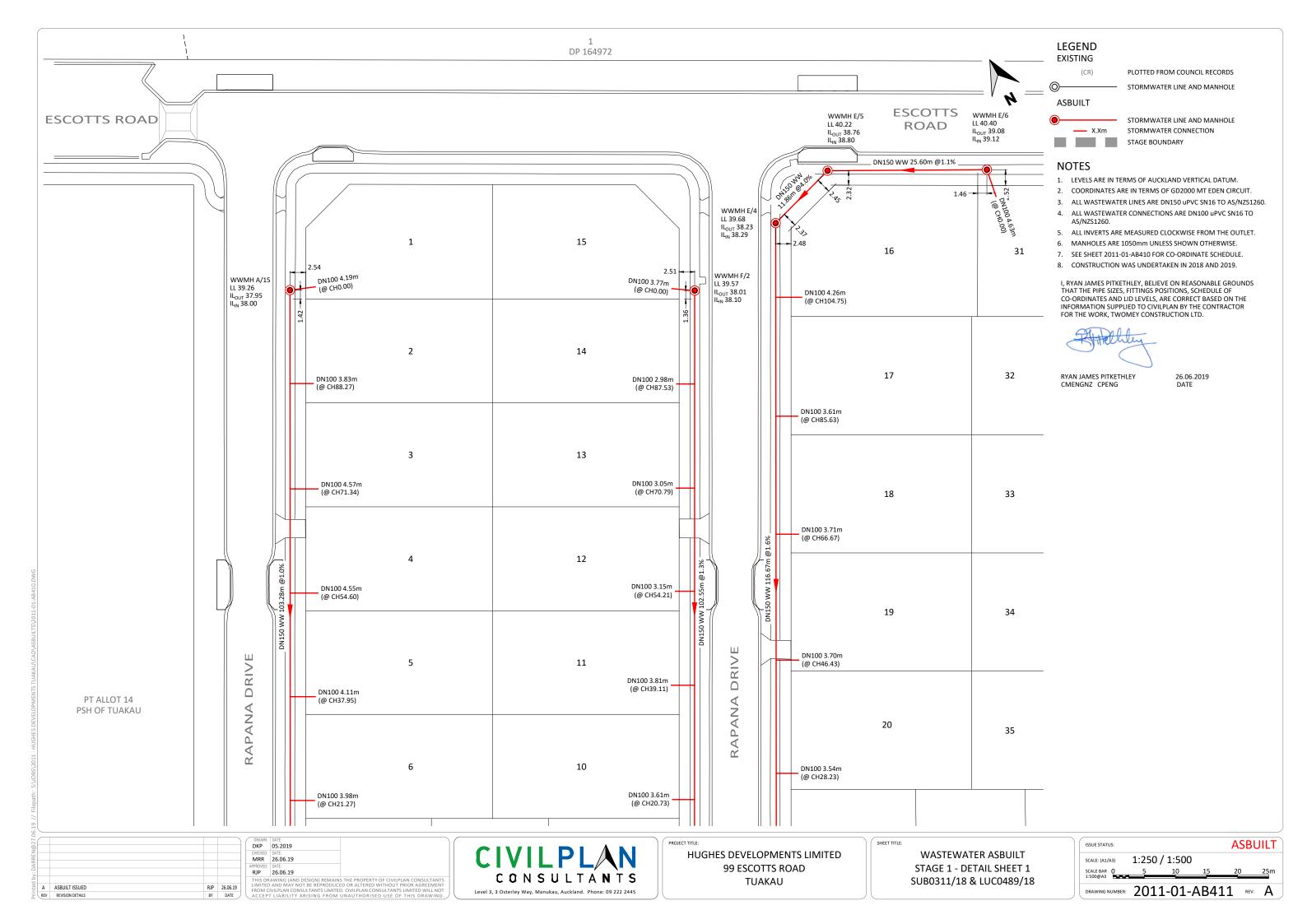
PROJECT TITLE:

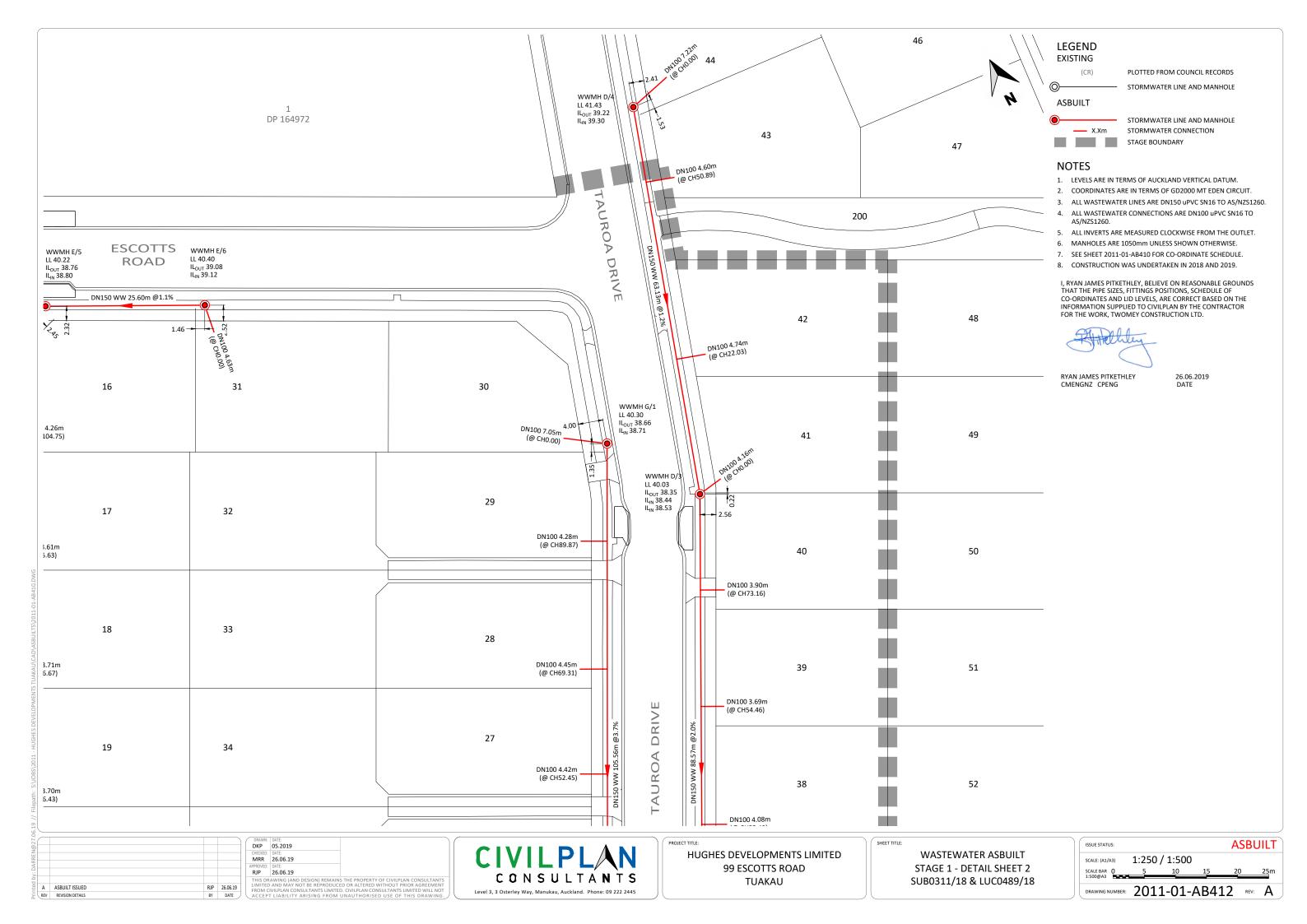
HUGHES DEVELOPMENTS LIMITED 99 ESCOTTS ROAD TUAKAU SHEET III

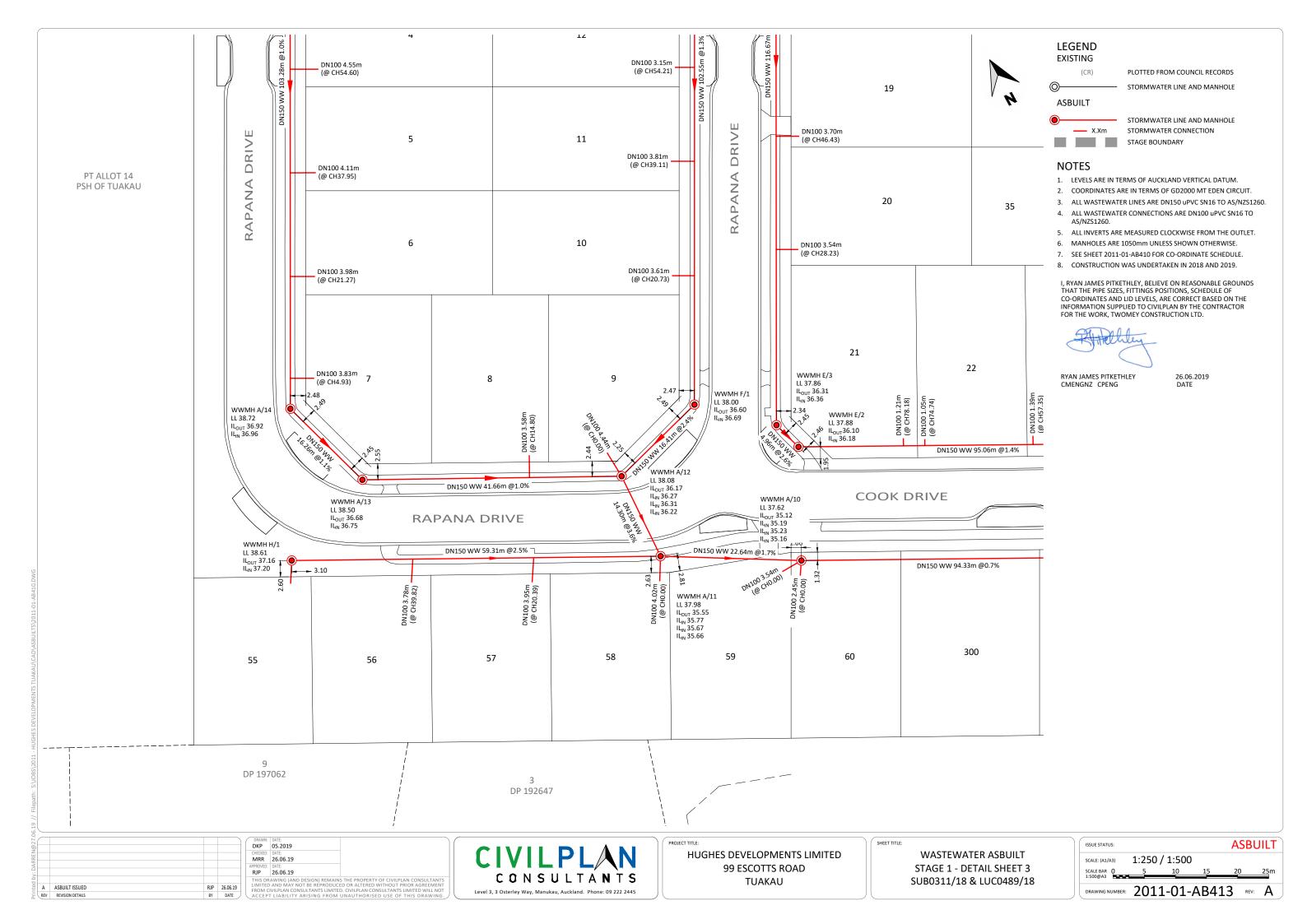
STORMWATER ASBUILT STAGE 1 - RAIN GARDEN - DETAIL SHEET 3 SUB0311/18 & LUC0489/18

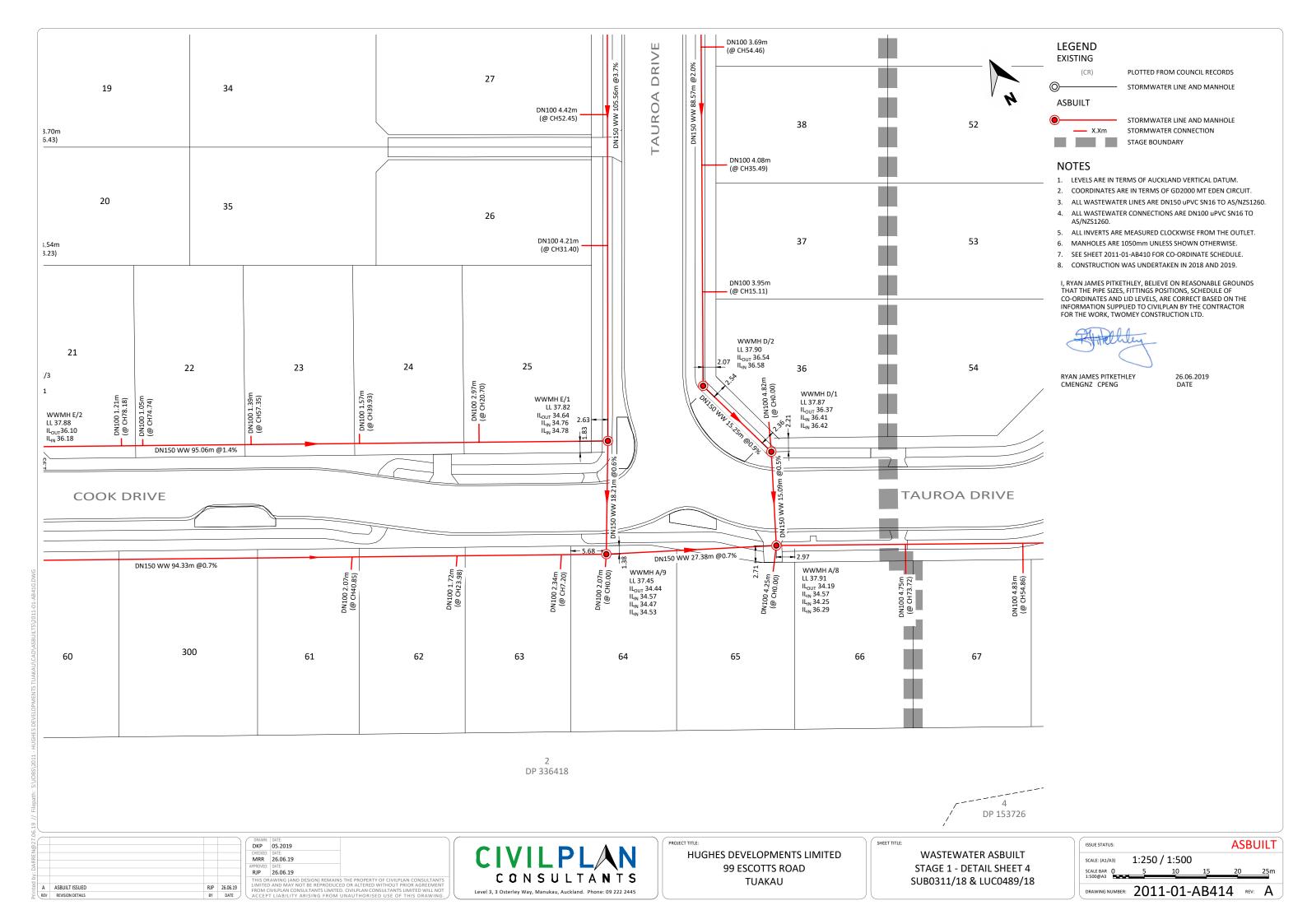


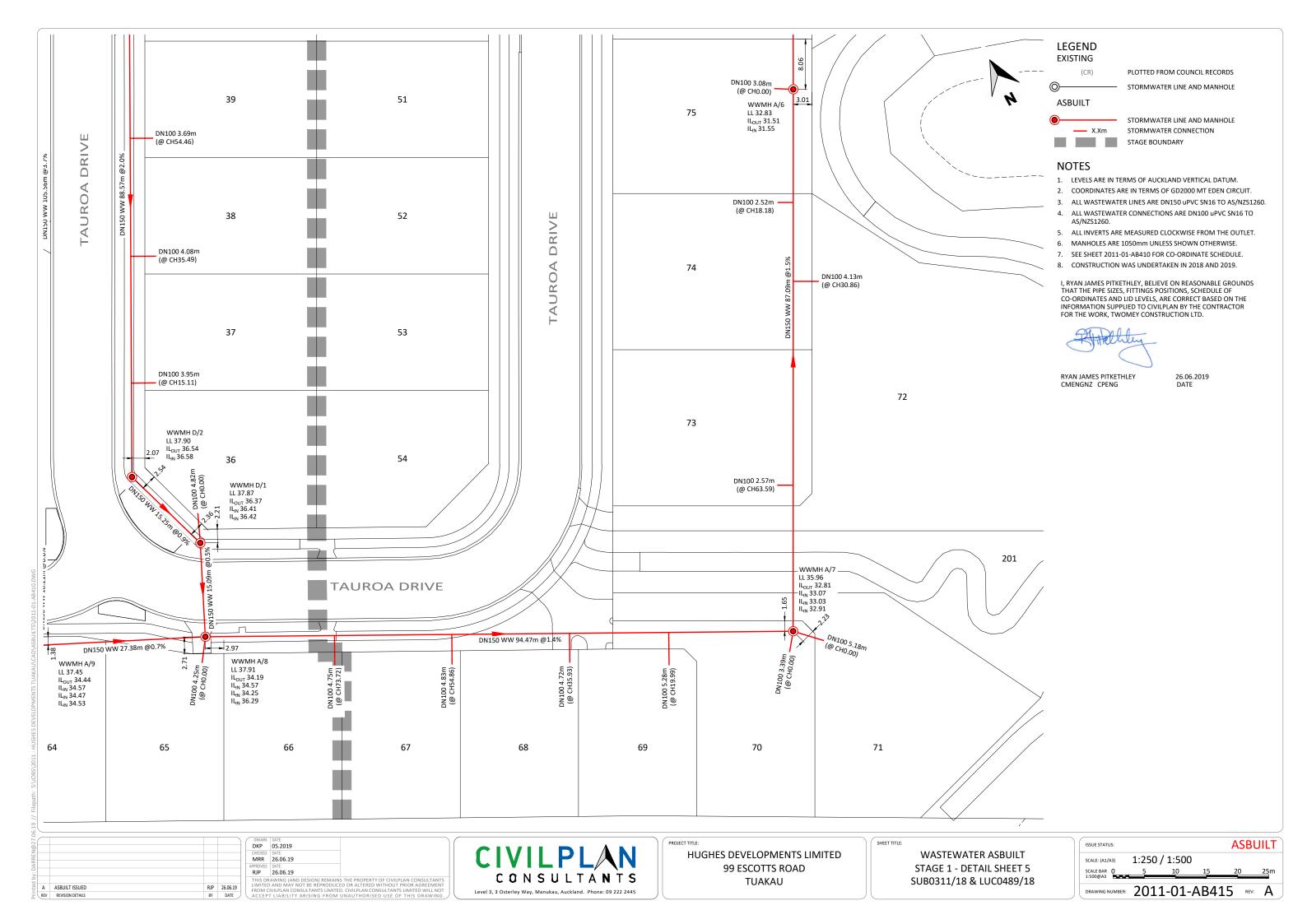


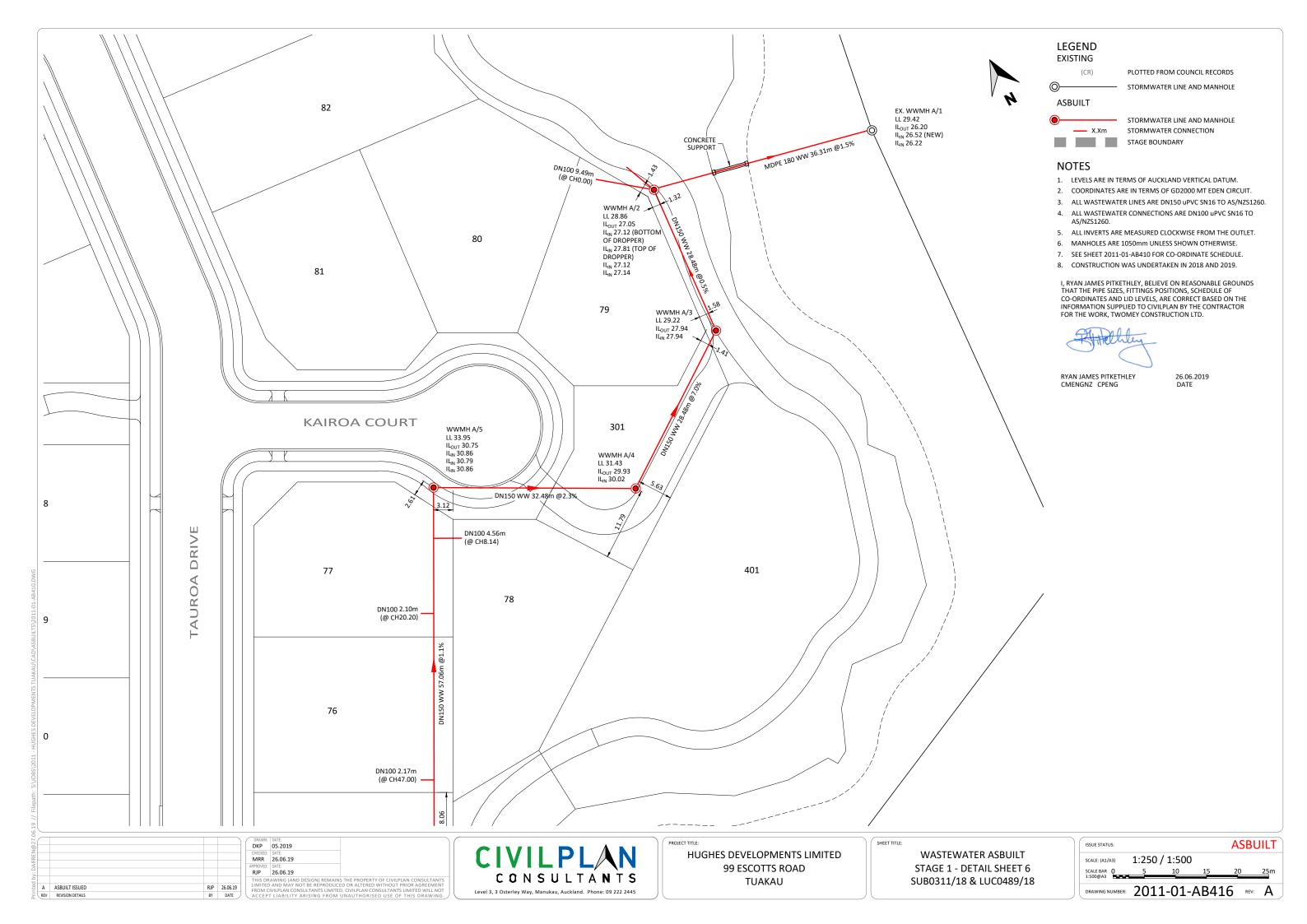


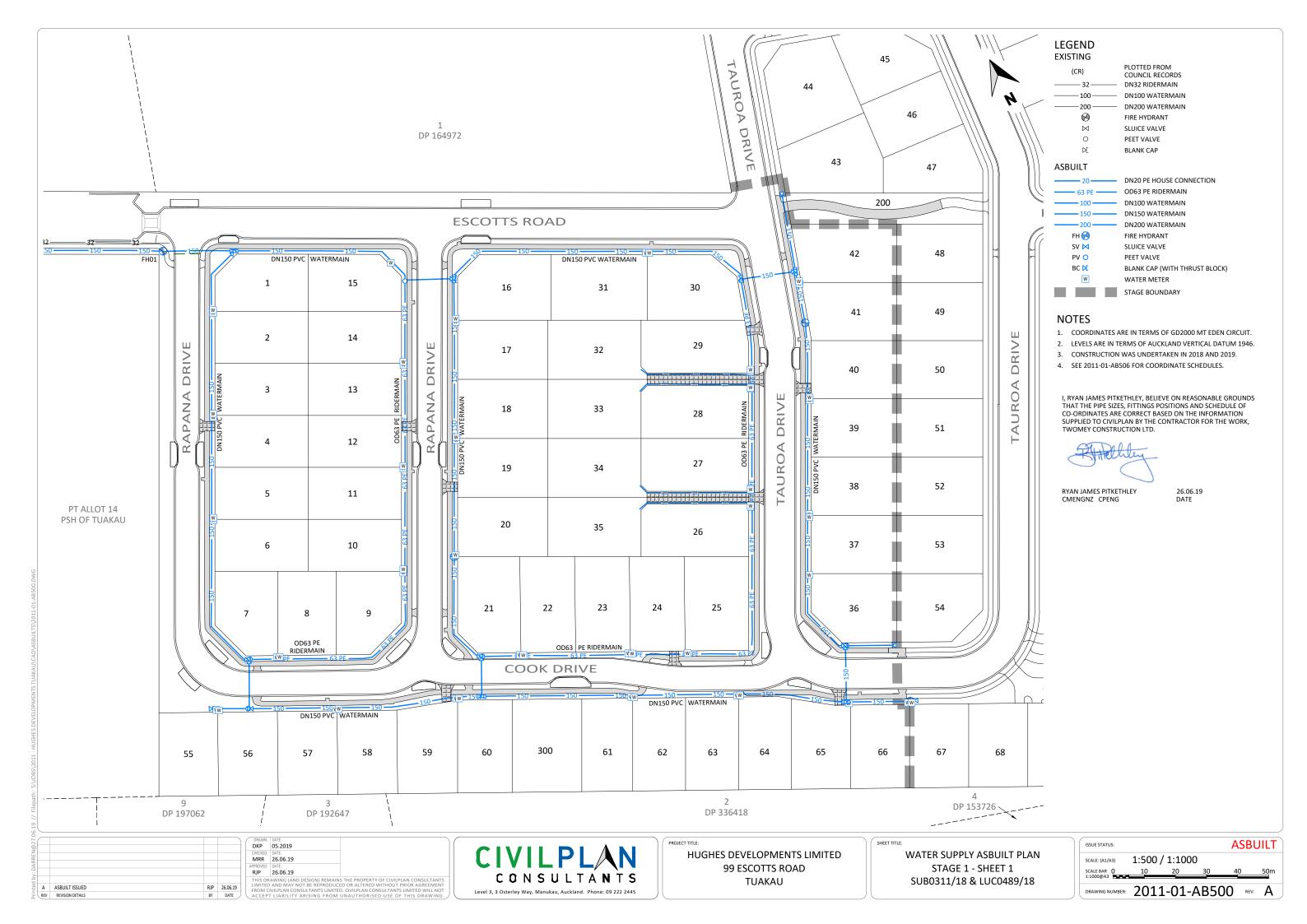


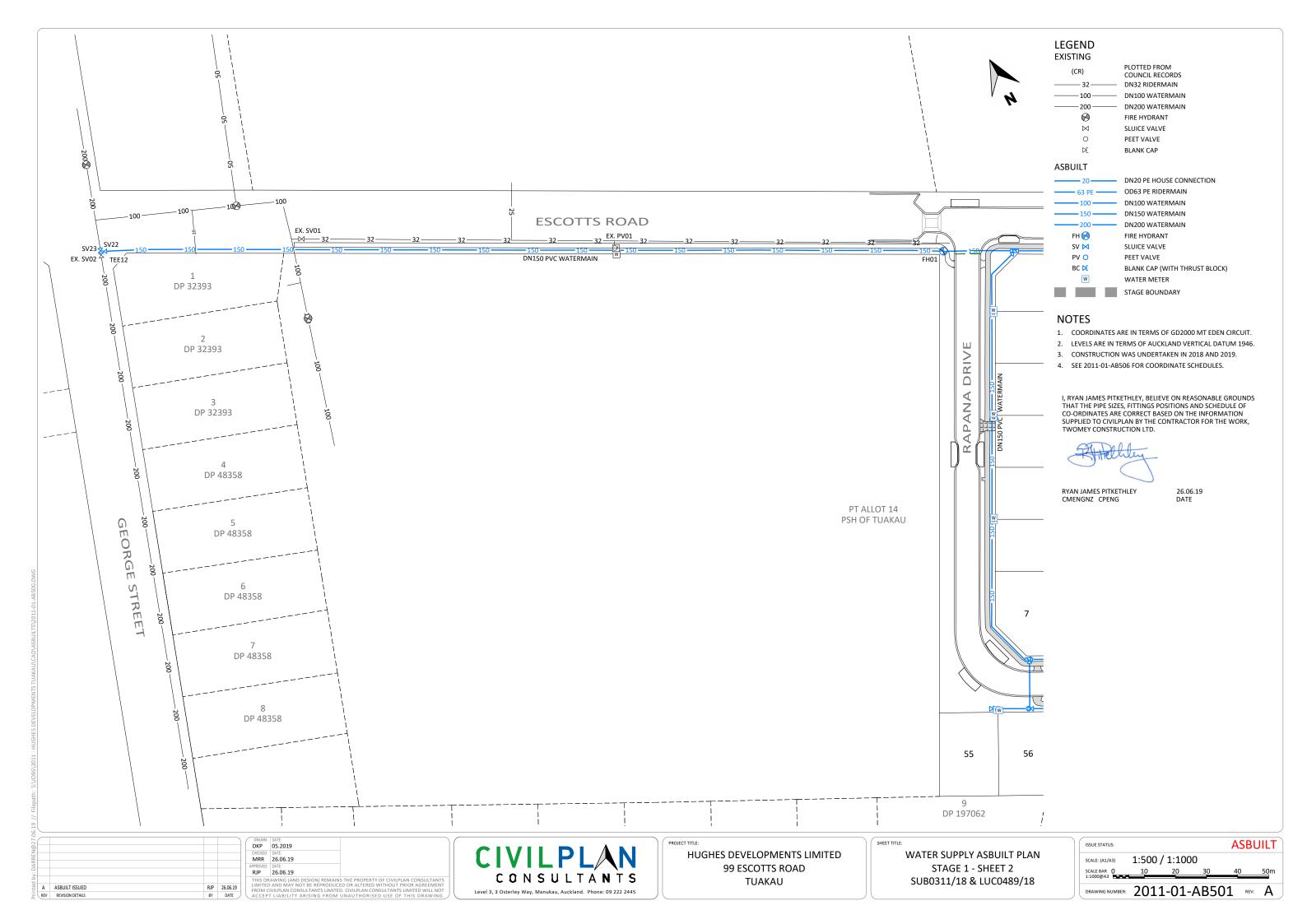


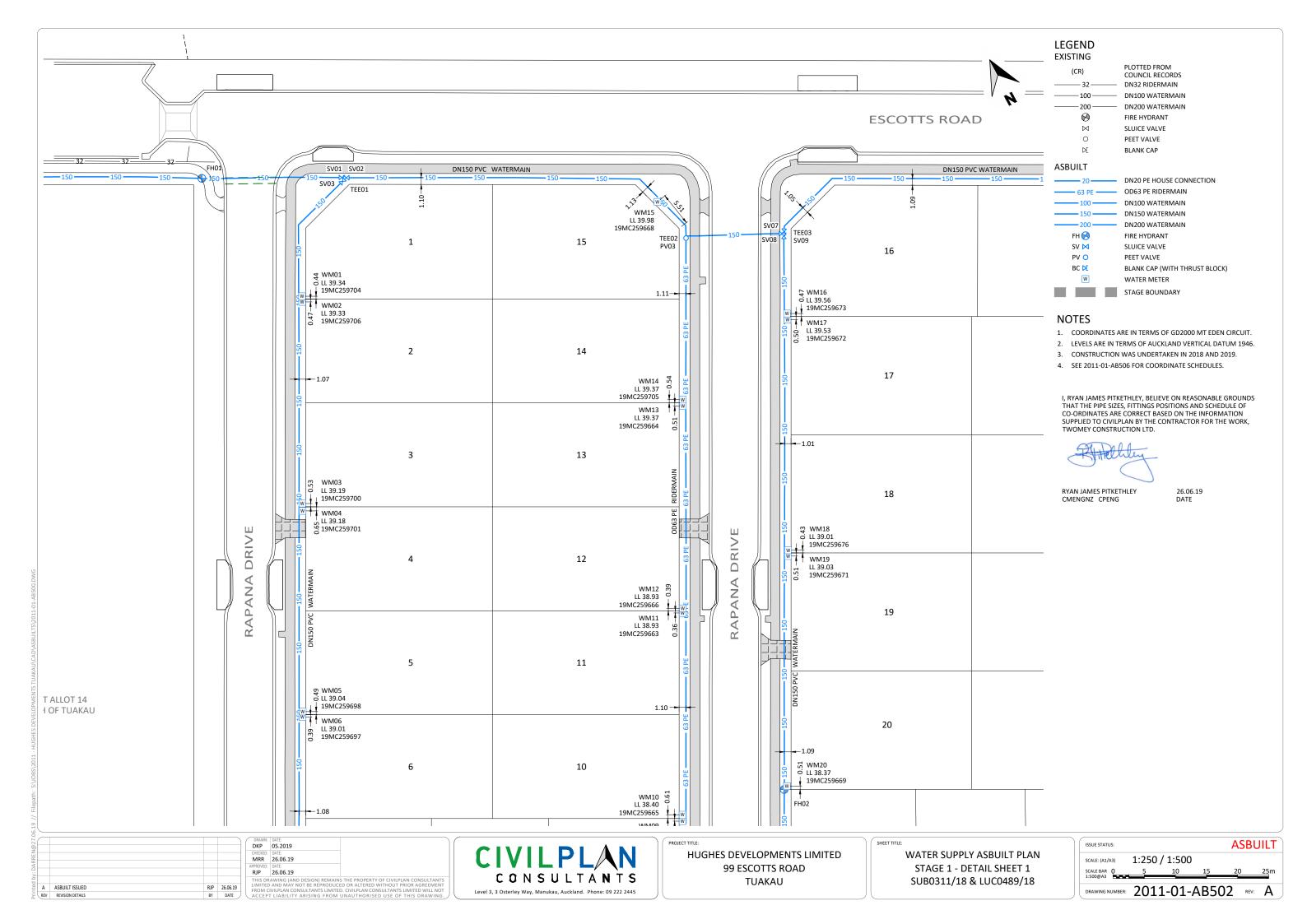


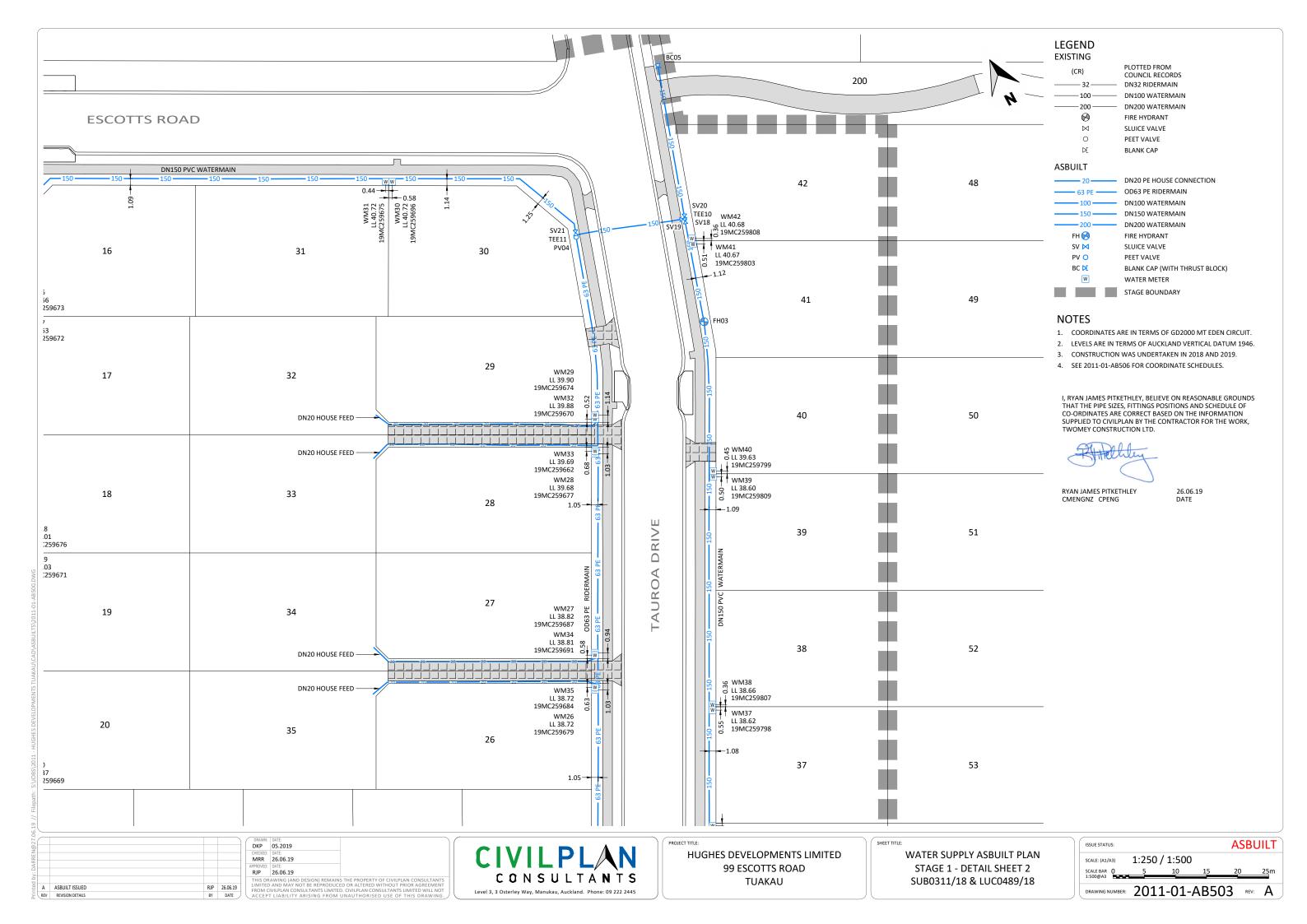


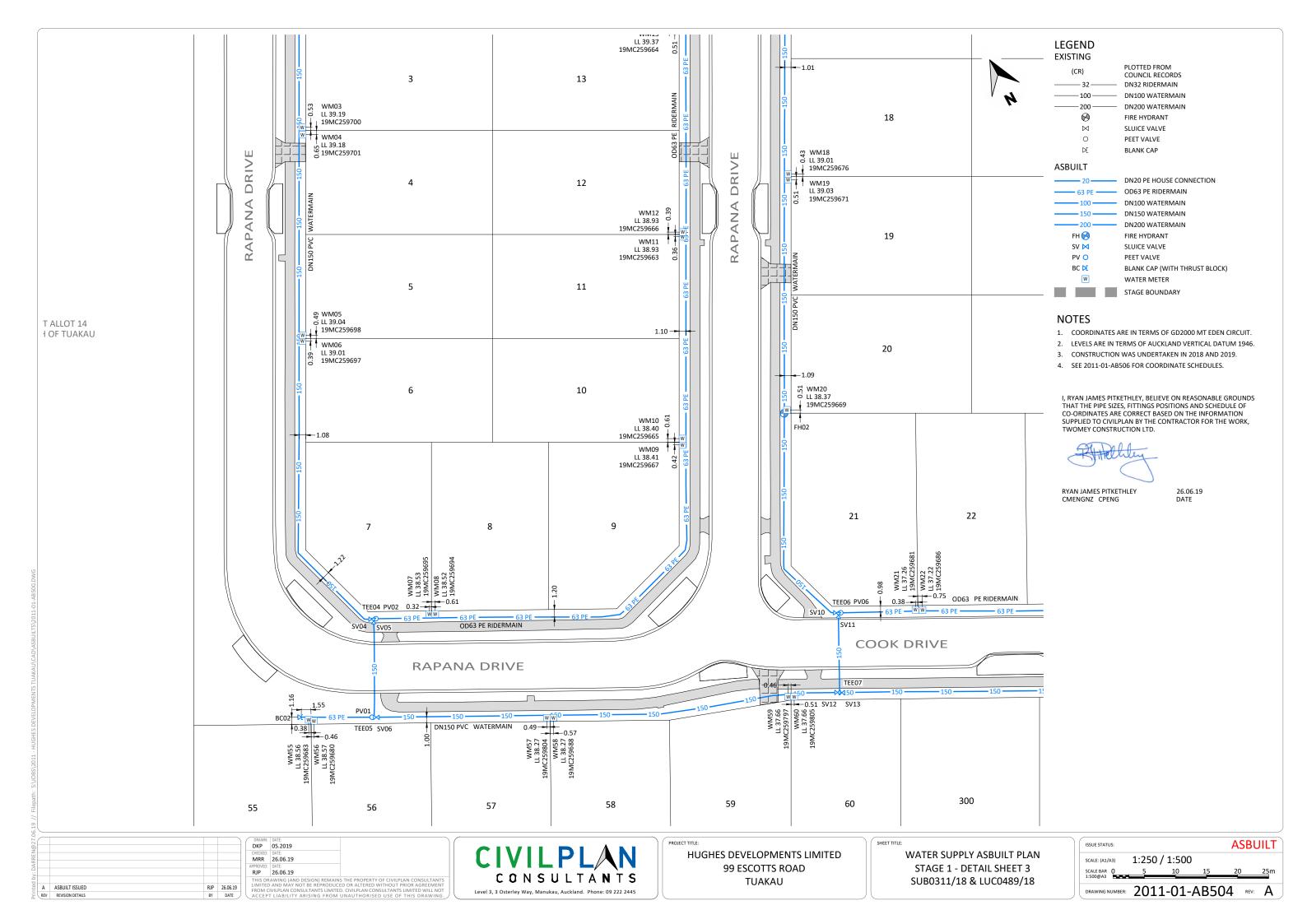


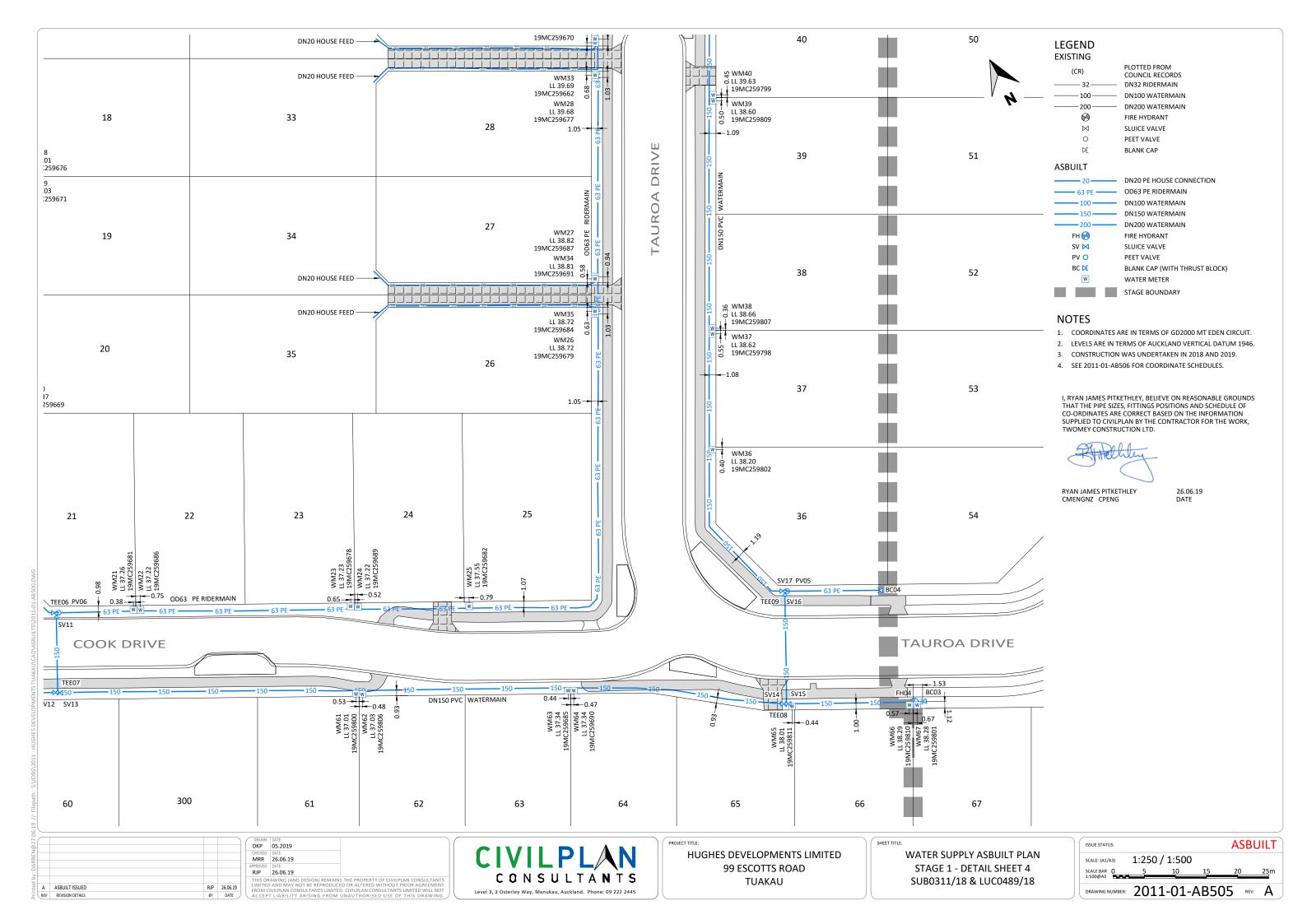












SCHEDULE OF COORDINATES PEET VALVES-GD2000				
POINT No. mE mN				
EX. PV01	416143.80	757308.68		
PV01	416191.37	757115.40		
PV02	416199.36	757129.09		
PV03	416300.62	757212.90		
PV04	416396.00	757162.16		
PV05	416370.64	757042.63		
PV06	416265.71	757094.73		

SCHEDULE OF COORDINATES FIRE HYDRANTS-GD2000			
mE	mN		
416236.51	757258.03		
416272.80	757127.29		
416407.74	757140.46		
416380.26	757017.28		
	HYDRANTS-0 mE 416236.51 416272.80 416407.74		

SCHEDULE OF COORDINATES TEES-GD2000					
POINT No. mE mN					
TEE01	416256.74	757247.37			
TEE02	416300.81	757213.20			
TEE03	416314.90	757206.09			
TEE04	416199.06	757129.22			
TEE05	416191.66	757115.23			
TEE06	416265.40	757094.89			
TEE07	416259.53	757083.50			
TEE08	416361.98	757026.70			
TEE09	416370.33	757042.79			
TEE10	416412.84	757156.51			
TEE11	416396.14	757162.48			
TEE12	415997.48	757385.42			

SCHEDULE OF COORDINATES			
BLANK CAPS-GD2000			
POINT No.	mE	mN	
BC02	416181.35	757120.77	
BC03	416381.48	757016.75	
BC04	416383.73	757035.79	
BC05	416420.50	757179.99	

NOTES

- 1. COORDINATES ARE IN TERMS OF GD2000 MT EDEN CIRCUIT.
- 2. LEVELS ARE IN TERMS OF AUCKLAND VERTICAL DATUM 1946.
- 3. CONSTRUCTION WAS UNDERTAKEN IN 2018 AND 2019.

I, RYAN JAMES PITKETHLEY, BELIEVE ON REASONABLE GROUNDS THAT THE PIPE SIZES, FITTINGS POSITIONS AND SCHEDULE OF CO-ORDINATES ARE CORRECT BASED ON THE INFORMATION SUPPLIED TO CIVILPLAN BY THE CONTRACTOR FOR THE WORK, TWOMEY CONSTRUCTION LTD.

RYAN JAMES PITKETHLEY CMENGNZ CPENG

26.06.19 DATE

DKP 05.2019 APPROVED: DATE: RJP 26.06.19 REV REVISION DETAILS

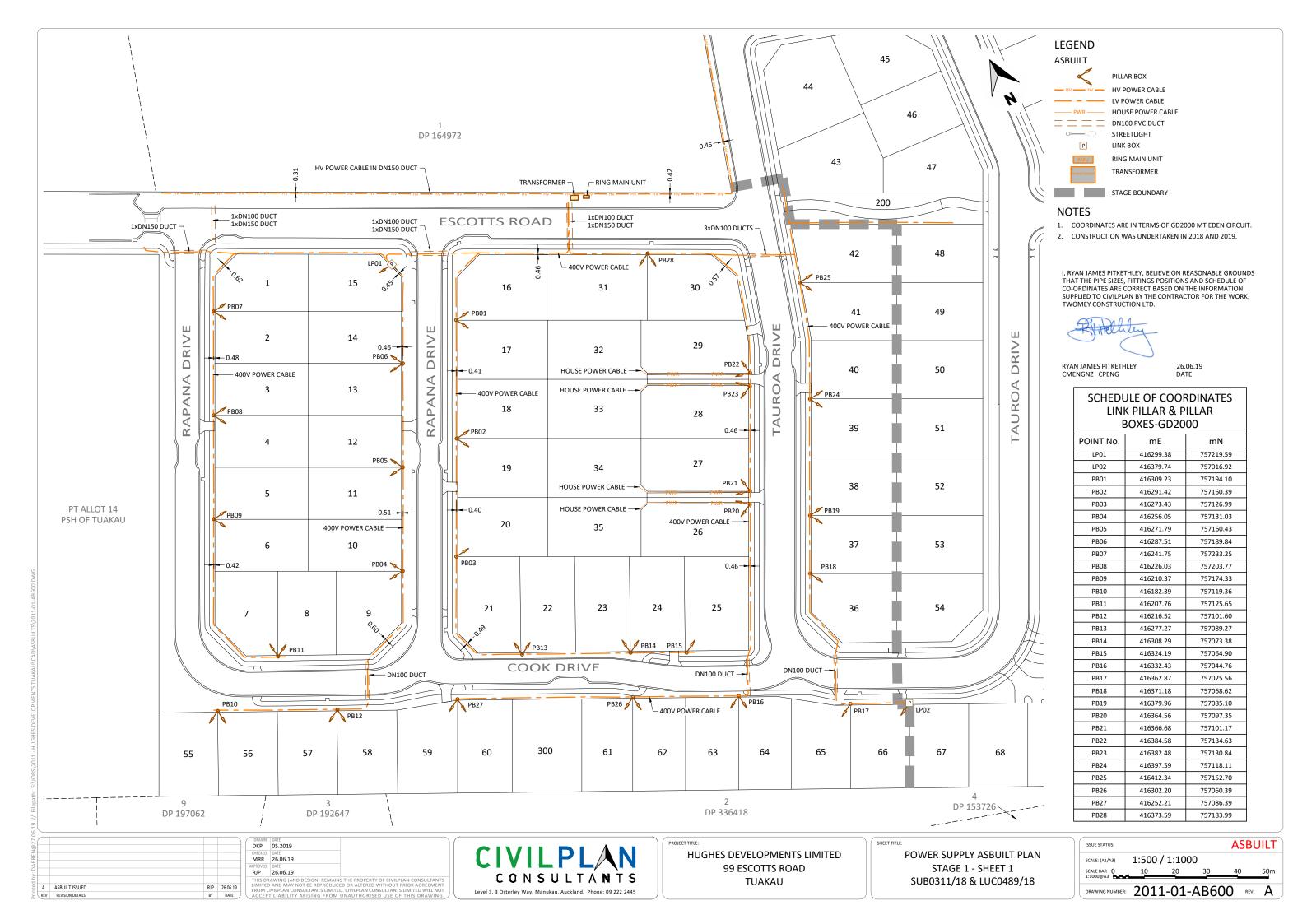


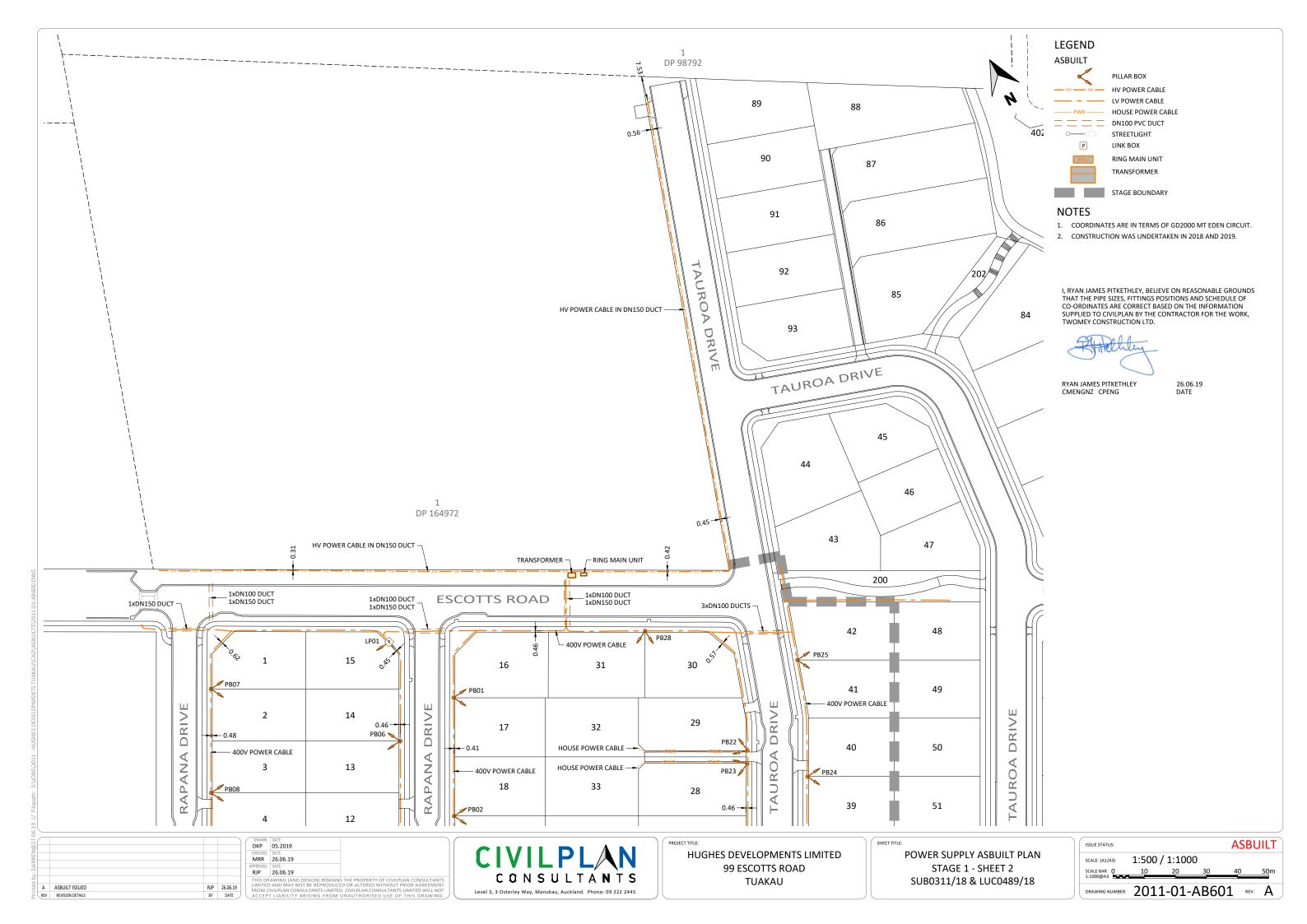


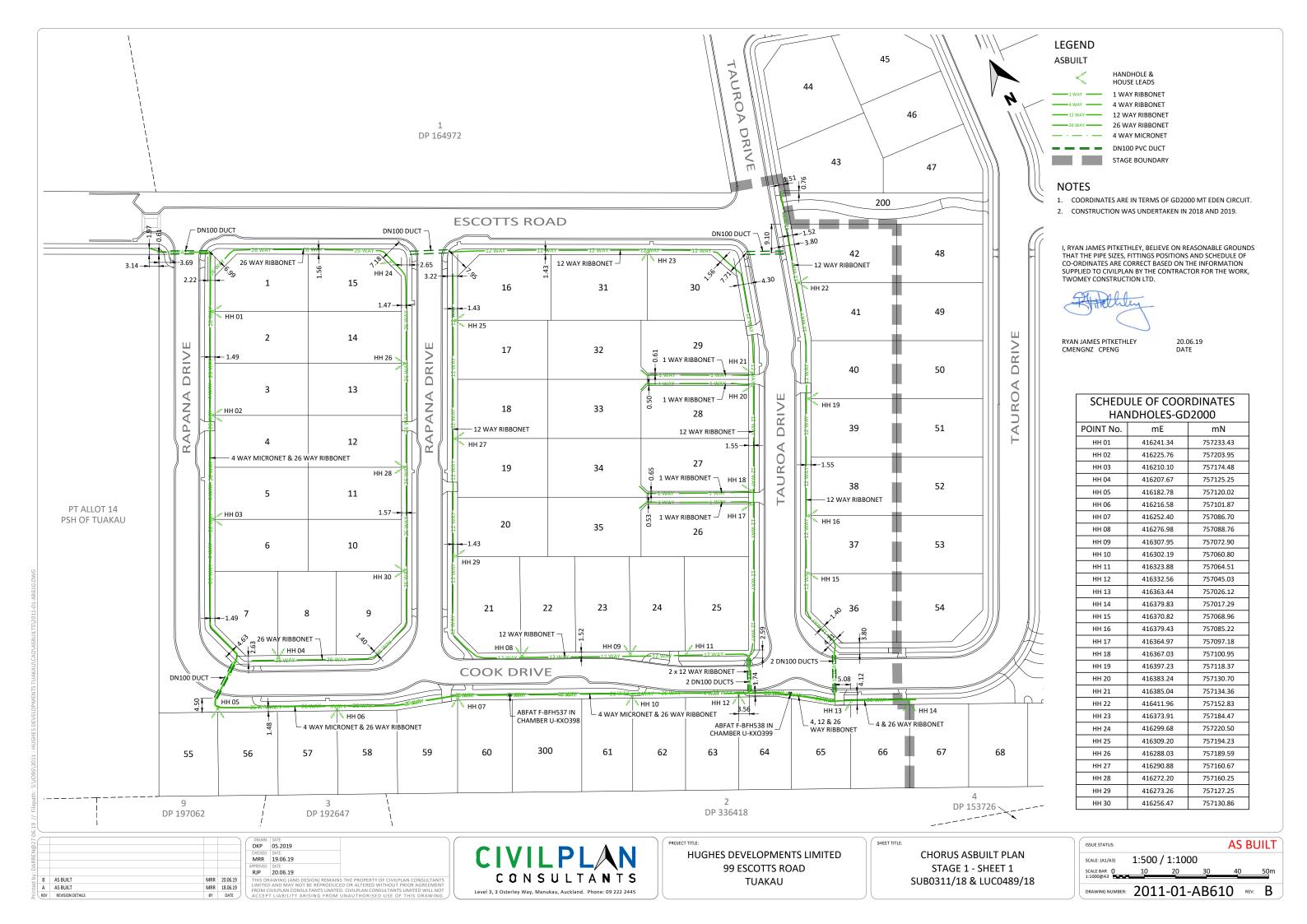
HUGHES DEVELOPMENTS LIMITED 99 ESCOTTS ROAD TUAKAU

WATER SUPPLY ASBUILT PLAN STAGE 1 - COORDINATE SCHEDULES SUB0311/18 & LUC0489/18

ISSUE STATUS:		ASBL	JIL٦
SCALE: (A1/A3) SCALE BAR N.T.S.	NOT TO SCALE		
	2011-01-AB506	REV:	_ A







Appendix 2

Contractor's PS3 (Construction)

Lander Geotechnical Consultants Limited PS4 (Construction Review)

SIXTH SCHEDULE

(NZS 3910:2003)

FORM OF PRODUCER STATEMENT CONSTRUCTION

ISSUED BY	ICB Retaining & Construction Limited
	(Contractor)
то	Twomey Construction Ltd
	(Principal)
IN RESPECT OF	Steel UB and Timber Pole Retaining Walls,
	Wall 1,2,3 at 99 Escotts Road Tuakau, Auckland
	(Description of Contract Works)
AT	99 Escotts Road Tuakau, Auckland
	(Address)
	ICB Retaining & Construction Ltd
	(Contractor)
has contracted to	, ,
	Twomey Construction Ltd (Principal)
to carry out and complete certain by	uilding works in accordance with a contract, titled
Supply and Installation of S	•
Retaining Walls, Wall 1,2,3 a	
	ents limited (The Contract)
(The Pro	
I, Regai	a duly authorised
(Duly Authoris	sed Agent)
representative of IC	CB Retaining & Construction Limited
1 Cpresentative of	(Contractor)
Policy on represents arrange that	TOP Poteining 9 Construction Limited
believe on reasonable grounds that	ICB Retaining & Construction Limited
	(Contractor)
has carried out and completed:	
	in the attached particulars of the building works in
Variations that have been issued du	ent (BLD 0360/19) and any Authorised Instruction /
	This the course of the work.
	(Signature of Authorised Agent on Behalf of)
	29 January 2019
	(Date)
	ICB Construction Limited
	(Contractor)
	DO Poy 202 240 North Harbour Augland
	PO Box 303 340, North Harbour, Auckland (Address)
	, · · · · · · · · · · · · · · · · · · ·



13 June 2019 Ref No: J00779

Hughes Developments Limited C/- CivilPlan Consultants Limited PO Box 97796 Auckland 2241

Attention: Mr R Pitkethley

Dear Ryan

RE: Producer Statement – Construction Review (PS4) for Walls 1 to 3, 99 Escotts Road, Tuakau

This is to confirm Lander Geotechnical Consultants Limited visited the above site on several occasions to observe ground conditions and retaining wall construction for Walls 1 to 3 of the Geotechnical Design Report (GDR; reference J00779, dated 9 May 2018).

Wall 1 construction commenced in late November 2018 with the trenching excavation of the basalt floaters in the alignment of the wall between points W1.3 to W1.5 (pre-emptively, as pile boring was unlikely to penetrate through the larger boulders). The trench excavations encountered a mantle of weathered silty clays (volcanic ash deposits) and loose rubbly basalt beneath this. The trench extended to the full embedment depths of the piles and was backfilled with engineer-certified clay fill. No significant groundwater was noted within the trench excavation. Following this, pile boring commenced, and the piles were all installed by early January 2019. Wall 1 was lagged and backfilled in February 2019.

Construction of Walls 2 and 3 commenced concurrently in January 2019 with the mucking out of the gully in the approximate alignment of the walls. Groundwater seepages were observed in the invert of the gully and an underfill drain and rotten rock blanket was placed and compacted here. A sump pit was formed and pumped/dewatered while the gully was filled with engineer-certified clay fill. Pile boring for both walls subsequently commenced and several of the holes were cased to prevent collapse due to the groundwater conditions at the base of the wall. Both walls were lagged and backfilled by late May 2019.

All clay fills associated with the undercuts along the wall alignments are presented in Appendix 4 of the overarching Geotechnical Completion Report for Stage 1, 99 Escotts Road.

Based on our site observations, we are satisfied that Walls 1 to 3 were constructed in accordance with the wall designs outlined in the Geotechnical Design Report (reference J00779 dated 9 May 2018) and that the ground conditions for the walls are in keeping with the assumed design parameters.

Phone: (09) 262 1528; (09) 262 1526 Email: contactus@landergeotechnical.co.nz



If you have any queries or require further information, please do not hesitate to contact the undersigned.

For and on behalf of Lander Geotechnical Consultants Limited

Prepared By:

Michael Chan

Geotechnical Project Engineer

NZDE(Civil)

Reviewed and Authorised By:

Chris Edwards

Senior Engineering Geologist

CMEngNZ(PEngGeol)







	81
Building Code Clause(s)	, i

PRODUCER STATEMENT – PS4 – CONSTRUCTION REVIEW

(Guidance on use of Producer Statements (formerly page 2) is available at www.engineeringnz.org)

ISSUED BY: Lander Geotechnical Consultants Limited (Construction Review Firm)
TO: Hughes Developments Limited
(Owner/Developer) TO BE SUPPLIED TO: Waikato District Council (Building Consent Authority)
IN RESPECT OF: Retaining Walls 1, 2 and 3 outlined in Lander Geotechnical Design Report dated 9 May 2019 ref. J00779 (Description of Building Work)
AT: 99 Escotts Road, Tuakau (Address)
Town/City: Waikato LOT. 1 DP. 169701 SO
We Lander Geotechnical Consultants Limited have been engaged by Hughes Developments Limited (Construction Review Firm)
To provide CM1 CM2 CM3 CM4 CM5 (Engineering Categories) or observation as per agreement with
owner/developer. Hughes Developments Limited
or other services (Extent of Engagement)
in respect of clause(s)
documents relating to Building Consent No. BLD0360/19 and those relating to
Building Consent Amendment(s) Nos issued during the course of the works. We have sighted these Building Consents and the conditions of attached to them.
Authorised instructions/variations(s) No
On the basis ofthis reviewthese review(s) and information supplied by the contractor during the course of the works and on behalf of the firm undertaking this Construction Review, I believe on reasonable grounds thatAll orPart only of the building works have been completed in accordance with the relevant requirements of the
Building Consent and Building Consent Amendments identified above, with respect to Clause(s). B1
I, Shane Lander am: CPEng 219353 # Reg Arch # Reg Arch #
I am a member of: Engineering New Zealand NZIA and hold the following qualifications BE(Hons), NZCE The Construction Review Firm issuing this statement holds a current policy of Professional Indemnity Insurance no less than \$200,000*.
The Construction Review Firm is a member of ACENZ:
SIGNED BY Shane Lander (Signature). (Signature).
SIGNED BY Shane Lander (Name of Construction Review Professional) ON BEHALF OF Lander Geotechnical Consultants Limited (Construction Review Firm) (Signature) Date 17/06/19

Note: This statement shall only be relied upon by the Building Consent Authority named above. Liability under this statement accrues to the Design Firm only. The total maximum amount of damages payable arising from this statement and all other statements provided to the Building Consent Authority in relation to this building work, whether in contract, tort or otherwise (including negligence), is limited to the sum of \$200,000*.

This form is to accompany Forms 6 or 8 of the Building (Form) Regulations 2004 for the issue of a Code Compliance Certificate.

THIS FORM AND ITS CONDITIONS ARE COPYRIGHT TO ACENZ, ENGINEERING NEW ZEALAND AND NZIA

Appendix 3

Classification Test Data



Our Ref: 1009521.0220/Rep1 Customer Ref: J00779 17 April 2019

Lander Geotechnical Consultants Limited Level 3 3 Osterley Way Manukau 2104

Attention: Shane Lander

99 Escotts Road, Tuakau Laboratory Test Report

Samples from the above mentioned site have been tested as received according to your instructions. Test results are included in this report.

Samples not destroyed during testing will be retained for one month from the date of this report before being discarded.

Descriptions are enclosed for your information, but are not covered under the IANZ endorsement of this report.

Please reproduce this report in full when transmitting to others or including in internal reports.

If we can be of any further assistance, feel free to get in touch. Contact details are provided at the bottom of this page.

GEOTECHNICS LTD

Report prepared by:

Authorised for Geotechnics by:

Brendon Kingham
Laboratory Technician
Approved Signatory

Authorised for Geotechnics by:

Paul Burton
Project Director

Report checked by:

HM()

ACCREDITED LABORATORY

Tests indicated as not accredited are outside the scope of the laboratory's accreditation

Ryan Milligan Project Manager

17-Apr-19

 $\label{lem:local-corporate-geotechnics} \label{lem:local-corporate-geotechnics} \lab$

Our Ref: 1009521.0220/Rep1



Tauranga 15C Amber Crescent Judea Tauranga 3110 New Zealand

p: +64 7 571 0280

Report No: MAT:S19TG000162 Issue No: 1

Material Test Report

Customer: Lander Geotechnical

Address:

Project: 99 Escotts Road Tuakau - Lab

Project No.: 1009521.0220

Customer Reference No.: J00779

Report Authorised By: Tylah Wardrope

ACCREDITED LABORATORY

Tests indicated as not accredited (#) are outside the scope of the laboratory's accreditation.

Tests indicated as not accredited (#) are outside the scope of the laboratory's accreditation.

Approved By:
Brendon Kingham
(Geotechnical Technician)
Date of Issue: 17/04/2019

Please reproduce this report in full when transmitting to others or including in internal reports.

Sample Details

Location 99 Escotts Road, Tuakau

Geotechnics ID S19TG000162

Sample Reference Lot 3

Sample Description SILT, with some clay and some sand, trace gravel; orange brown. Moist.

Sample Depth 0.50m Bottom Depth 1.0m

Test Results

Description	Method	Result	Limits
Moisture Content (%)	NZS 4402:1986 Test 2.1	52.8	
Date Tested		12/04/2019	

Comments

N/A

If samples have been taken, and were not destroyed during testing, they will be retained for one month from the date of this report before being discarded.

Our Ref: 1009521.0220/Rep1

Form No: 18909, Report No: MAT:S19TG000162

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GEOTECHNICS

15C Amber Crescent

Judea

Tauranga 3110

New Zealand p: +64 7 571 0280

QESTLab Work Order ID

Geotechnics Project Number 1009521.0220

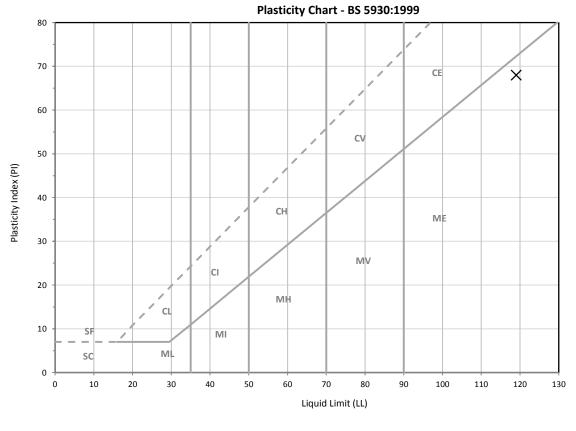
Customer Project ID J00779

Determination of Liquid & Plastic Limit, Plasticity Index - NZS 4402: 1986 Tests 2.2 (4 Point), 2.3 & 2.4

		TEST DETAILS			
LOCATION	Description	99 Escotts Road, Tuakau			
	Data	N/A			
SAMPLE	Geotechnics ID	S19TG000162			
	Reference	Lot 3	Top Depth	0.50m	
	Sampled By	Others, Tested As Received	Bottom Depth	1.0m	
	Description	SILT, with some clay and some	sand, trace gravel; orange	brown. Moist.	
SPECIMEN	Reference	N/A	Depth	N/A	
	Description	N/A			

TEST RESULTS

Liquid Limit 119
Plastic Limit 51
Plasticity Index 68



— A Line

Page 3 of 7

W19TG-0043

— — B Line

Soil Type M - Silt

C - Clay

S - Sand

<u>Plasticity</u>

L - Low

I - Intermediate

H - High

V - Very High

E - Extremely High

The plasticity chart is provided for your inference only and is not covered under our scope of IANZ accreditation. Due to the nature of classifications it is possible to have discrepancies between observational behaviour descriptions and measured parameters

Our Ref: 1009521.0220/Rep1

TEST REMARKS

• The material used for testing was natural, fraction passing a 425um sieve. • Results apply only to sample tested. • This report may be reproduced only in full.

Approved By Brendon Kingham

Date 17/04/2019



Tests indicated as not accredited (#) are outside the scope of the laboratory's accreditation.



15C Amber Crescent

Judea

Tauranga 3110 New Zealand

p: +64 7 571 0280

Geotechnics Project Number

QESTLab Work Order ID Customer Project ID

1009521.0220

Page 4 of 7

W19TG-0043

J00779

Determination of Linear Shrinkage - Determination of the Linear Shrinkage - NZS 4402:1986 Test 2.6

Description Data Geotechnics ID Reference	99 Escotts Road, Tuakau N/A S19TG000162 Lot 3	Tan Danith	
Geotechnics ID	S19TG000162	Too Doobb	
		Top Doubh	
Reference	Lot 3	Ton Douth	
		Top Depth	0.50m
Sampled By	Others, Tested As Received	Bottom Depth	1.0m
Description	SILT, with some clay and some sa	nd, trace gravel; orange brown.	Moist.
Reference		Depth	
Description			
			·

Linear Shrinkage

26%

TEST REMARKS

Our Ref: 1009521.0220/Rep1

• Results apply only to sample tested. • This report may be reproduced only in full.

Approved By Brendon Kingham 17/04/2019



Tests indicated as not accredited (#) are outside the scope of the laboratory's accreditation.

Date

Issue No: 1



Tauranga 15C Amber Crescent Judea Tauranga 3110 New Zealand

p: +64 7 571 0280

Material Test Report

Customer: Lander Geotechnical

Address:

Project: 99 Escotts Road Tuakau - Lab

Project No.: 1009521.0220

Customer Reference No.: J00779

Report Authorised By: Tylah Wardrope ACCREDITED LABORATORY

Tests indicated as not accredited (#) are outside the scope of the laboratory's accreditation

Report No: MAT:S19TG000163

Tests indicated as not accredited (#) are outside the scope of the laboratory's accreditation.

Approved By: Brendon Kingham (Geotechnical Technician) 17/04/2019 Date of Issue:

Please reproduce this report in full when transmitting to others or including in internal reports.

Sample Details

Location 99 Escotts Road, Tuakau

Geotechnics ID S19TG000163

Sample Reference Lot24

Sample Description SILT, with some sand, minor to some clay, trace gravel and trace rootelts; orange brown. Moist.

Sample Depth 0.50m **Bottom Depth** 1.0m

Test Results

Description	Method	Result L	imits.
Moisture Content (%)	NZS 4402:1986 Test 2.1	46.4	
Date Tested		12/04/2019	

Comments

N/A

If samples have been taken, and were not destroyed during testing, they will be retained for one month from the date of this report before being discarded.

Form No: 18909, Report No: MAT:S19TG000163

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Page 1 of 1

15C Amber Crescent

Judea

Tauranga 3110

New Zealand p: +64 7 571 0280 **Geotechnics Project Number**

1009521.0220

QESTLab Work Order ID

W19TG-0043

Page 6 of 7

Customer Project ID

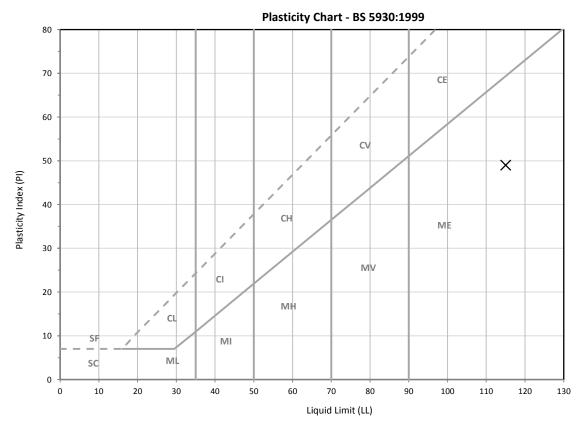
J00779

Determination of Liquid & Plastic Limit, Plasticity Index - NZS 4402: 1986 Tests 2.2 (4 Point), 2.3 & 2.4

Description	99 Escotts Road, Tuakau		
Data	N/A		
Geotechnics ID	S19TG000163		
Reference	Lot24	Top Depth	0.50m
Sampled By	Others, Tested As Received	Bottom Depth	1.0m
Description	SILT, with some sand, minor to	some clay, trace gravel and	d trace rootelts; orange brown. Mois
Reference	N/A	Depth	N/A
Description	N/A		
	Data Geotechnics ID Reference Sampled By Description Reference	Data N/A Geotechnics ID S19TG000163 Reference Lot24 Sampled By Others, Tested As Received Description SILT, with some sand, minor to	Data N/A Geotechnics ID S19TG000163 Reference Lot24 Top Depth Sampled By Others, Tested As Received Bottom Depth Description SILT, with some sand, minor to some clay, trace gravel and the same sand.

TEST RESULTS

Liquid Limit 115 Plastic Limit 66 Plasticity Index 49



A Line

— B Line

Soil Type M - Silt

C - Clay S - Sand

Plasticity

L - Low

I - Intermediate

H - High

V - Very High

E - Extremely High

The plasticity chart is provided for your inference only and is not covered under our scope of IANZ accreditation. Due to the nature of classifications it is possible to have discrepancies between observational behaviour descriptions and measured parameters

Our Ref: 1009521.0220/Rep1

TEST REMARKS

• The material used for testing was natural, fraction passing a 425um sieve. • Results apply only to sample tested. • This report may be reproduced only in full.

Approved By Brendon Kingham

17/04/2019



Tests indicated as not accredited (#) are outside the scope of the laboratory's accreditation.

Date



15C Amber Crescent

Judea

Tauranga 3110 New Zealand

p: +64 7 571 0280

Geotechnics Project Number

QESTLab Work Order ID

1009521.0220 W19TG-0043

Page 7 of 7

Customer Project ID

J00779

Determination of Linear Shrinkage - Determination of the Linear Shrinkage - NZS 4402:1986 Test 2.6

		TEST DETAILS							
LOCATION	Description	99 Escotts Road, Tuakau							
	Data	N/A							
SAMPLE	Geotechnics ID	S19TG000163							
	Reference	Lot24	Top Depth	0.50m					
	Sampled By	Others, Tested As Received	Bottom Depth	1.0m					
	Description	SILT, with some sand, minor to so	me clay, trace gravel and trace rootelts; orange brown. Moist.						
SPECIMEN	Reference		Depth						
	Description								

Linear Shrinkage

TEST REMARKS

Our Ref: 1009521.0220/Rep1

• Results apply only to sample tested. • This report may be reproduced only in full.

26%

ACCREDITED LABORAT

Tests indicated as not accredited (#) are outside the scope of the laboratory's accreditation.

Approved By Brendon Kingham

Date 17/04/2019

Appendix 4

Field Density Test Summary Sheets



Del.

Cesar Pura

ww.coffey.com

Client: Lander Geotechnical Consultants Limited

Address PO Box 97 385, Manukau 2241

Attention: Michael Chan

c.c:

Project: J00779 - 99 Escotts Road Tuakau

Location: Tuakau

PROJECT CODE: 773-ETAM00829AA

Page: 1 of 2

ACCREDITED LABORATORY

Tests indicated as not accredited are outside the scope of the laboratory's accreditation

Approved Signatory:

Issue date: 27/09/2018

Test Methods in accordance with: *Shear Strength (using field Shear vane in accordance with NZGS 2001): Nuclear Densometer Testing (in accordance with NZS 4407:2015 Test 4.2): Water Content Testing (in accordance with NZS 4402:1986 Test 2.1): Moisture contents

I	Test method:	and dry densities are			14200 2001)	. redolcar Born	Jonnete	in rooting (in do	001da1100 Will 1120 4401.2010 1	(iii	1 40001441100 Will 1420 4402	2.1000 10012.1). Wolstart	o dornorno
Ī		W 10 1 N	+ .				2			Field Shear Strength in kPa	Wet Density Oven Water			Air Voids (%)

Date	Work Order No: ETAM	Tested by	Test No.	Layer	Material tested	Location	Easting	Northing	RL (m)	Probe Test Depth (mm)	Comments	Field	Shear St	rength in	kPa		Oven Water Content (%)		Solid Density (T/m3) Assumed	Air Voids (%)
17/09/2018	ETAM18W04157	SC	1	Fill	Silty CLAY	As per plan	1772688	5873804	35.2	150		160	160	155	151	1.66	60.2	1.03	2.70	0
17/09/2018	ETAM18W04157	SC	2	Fill	Silty CLAY	As per plan	1772690	5873811	34.9	150		151	147	160	155	1.68	50.7	1.12	2.70	2



SITE PLAN

NOT TO SCALE

Project No: 773-ETAM00829AA

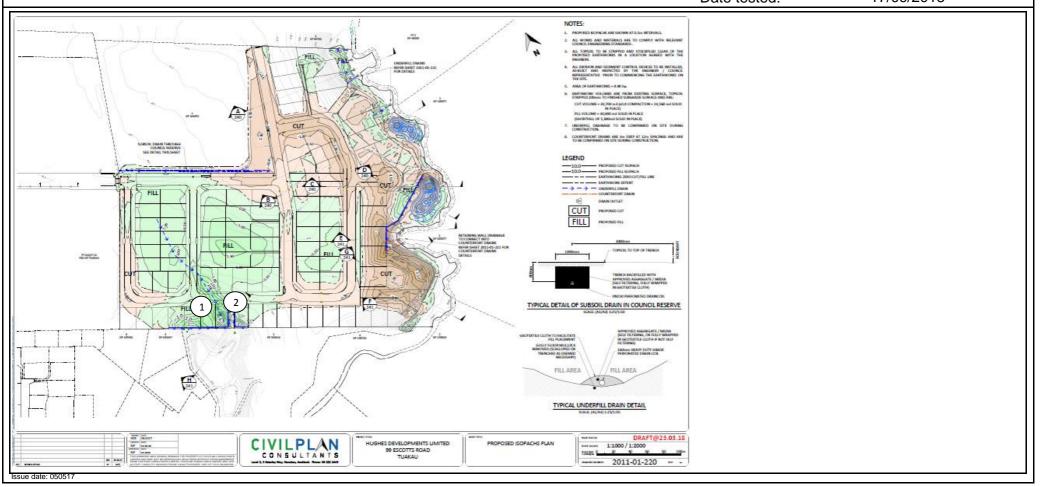
Work Order No: ETAM18W04157

Page No: 2 of 2

Project: J00779 - 99 Escotts Road Tuakau

Location: As below Tested by: SC

Date tested: 17/09/2018





201.

Cesar Pura

www.coffey.co

Client: Lander Geotechnical Consultants Limited

Address PO Box 97 385, Manukau 2241

Attention: Michael Chan

c.c:

Project: J00779 - 99 Escotts Road Tuakau

Location: Tuakau

PROJECT CODE: 773-ETAM00829AA

Page: 1 of 2

ACCREDITED LARGRATORY

Tests indicated as not accredited are outside the scope of the laboratory's accreditation

Approved Signatory:

Issue date: 31/10/2018

Test method: Test Methods in accordance with "Shear Strength (using field Shear vane in accordance with NZGS 2001): Nuclear Densometer Testing (in accordance with NZS 4407:2015 Test 4.2): Water Content Testing (in accordance with NZS 4402:1986 Test 2.1): Moisture contents and dry densities are corrected against oven dried moisture content testing.

and dry densities are corrected against oven dried moisture content testing.																				
Date	Work Order No: ETAM	Tested by	Test No.	Layer	Material tested	Location	Easting	Northing	RL (m)	Probe Test Depth (mm)	Comments	Field	d Shear S	trength in	kPa	Wet Density (T/m ³)	Oven Water Content (%)	Dry Density (T/m3)	Solid Density (T/m3) Assumed	Air Voids (%)
9/10/2018	ETAM18W04590	BS	3	Fill	Silty CLAY	Refer to Site Plan	1772674	5873934	-	150	1.0 m below Finished Level	UTP	UTP	UTP	UTP	1.60	39.2	1.15	2.70	12
9/10/2018	ETAM18W04590	BS	4	Fill	Silty CLAY	Refer to Site Plan	1772658	5873882	-	150	1.0 m below Finished Level	UTP	237+	237+	237+	1.57	56.1	1.00	2.70	6
9/10/2018	ETAM18W04590	BS	5	Fill	Silty CLAY	Refer to Site Plan	1772649	5873852	-	150	1.0 m below Finished Level	UTP	UTP	UTP	UTP	1.58	44.4	1.09	2.70	11
9/10/2018	ETAM18W04590	BS	6	Fill	Silty CLAY	Refer to Site Plan	1772672	5873874	-	150	1.0 m below Finished Level	UTP	UTP	UTP	UTP	1.62	53.3	1.05	2.70	5
9/10/2018	ETAM18W04590	BS	7	Fill	Silty CLAY	Refer to Site Plan	1772712	5873912	-	150	1.0 m below Finished Level	UTP	UTP	UTP	UTP	1.63	54.4	1.06	2.70	3
9/10/2018	ETAM18W04590	BS	8	Fill	Silty CLAY	Refer to Site Plan	1772706	5873873	-	150	1.0 m below Finished Level	UTP	UTP	UTP	UTP	1.70	43.3	1.19	2.70	5



Project:

SITE PLAN

Project No: 773-ETAM00829AA

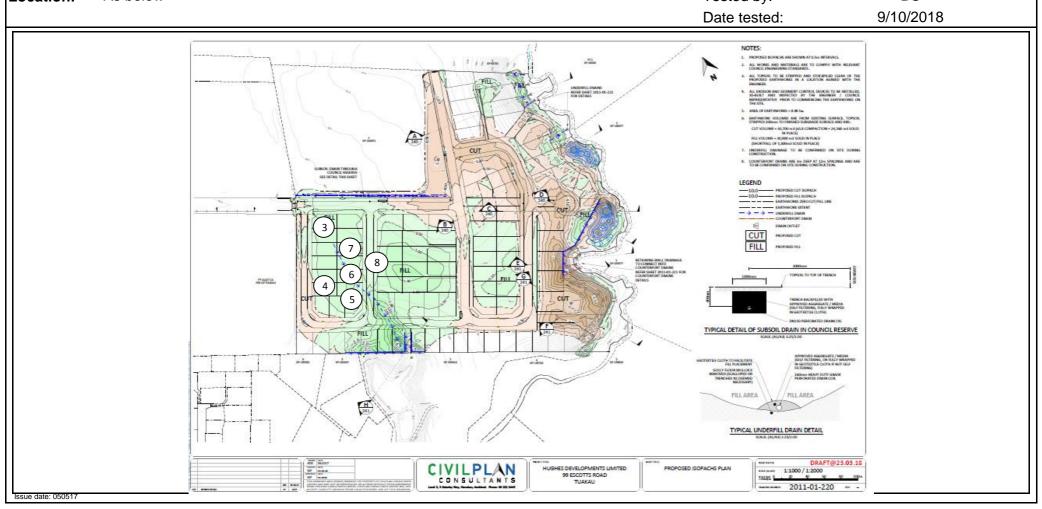
Work Order No: ETAM18W04590

Page No: 2 of 2

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J00779 - 99 Escotts Road Tuakau

Location: As below Tested by: BS





201.

www.coffey.co

Client: Lander Geotechnical Consultants Limited

Address PO Box 97 385, Manukau 2241

Attention: Michael Chan

c.c:

Project: J00779 - 99 Escotts Road Tuakau

Location: Tuakau

PROJECT CODE: 773-ETAM00829AA

Page: 1 of 2

ACCREDITED LABORATORY

Tests indicated as not accredited are outside the scope of the laboratory's accreditation

Approved Signatory:

Cesar Pura

Issue date: 31/10/2018

Test method: Test Methods in accordance with NZS 4407:2015 Test 4.2): Water Content Testing (in accordance with NZS 4402:1986 Test 2.1): Moisture contents and dry densities are corrected against oven dried moisture content testing.

	and dry densities are corrected against oven dried moisture content testing.																			
Date	Work Order No: ETAM	Tested by	Test No.	Layer	Material tested	Location	Easting	Northing	RL (m)	Probe Test Depth (mm)	Comments	Field	d Shear S	trength in	kPa	Wet Density (T/m ³)	Oven Water Content (%)	Dry Density (T/m3)	Solid Density (T/m3) Assumed	Air Voids (%)
10/10/201	B ETAM18W04592	BS	9	Fill	Silty CLAY	Refer to Site Plan	1772704	5873929	39.1	150	-	188	188	218	233	1.62	61.7	1.00	2.70	1
10/10/201	B ETAM18W04592	BS	10	Fill	Silty CLAY	Refer to Site Plan	1772679	5873874	38.7	150	-	237+	237+	218	UTP	1.60	60.6	1.00	2.70	3
10/10/201	B ETAM18W04592	BS	11	Fill	Silty CLAY	Refer to Site Plan	1772718	5873848	37.9	150	-	237+	237+	237+	237+	1.61	54.6	1.04	2.70	4
10/10/201	B ETAM18W04592	BS	12	Fill	Silty CLAY	Refer to Site Plan	1772751	5873902	39.1	150	-	237+	237+	237+	237+	1.60	45.2	1.10	2.70	9
10/10/201	B ETAM18W04592	BS	13	Fill	Silty CLAY	Refer to Site Plan	1772762	5873892	39.5	150	-	163	188	170	144	1.60	52.7	1.05	2.70	6
10/10/201	B ETAM18W04592	BS	14	Fill	Silty CLAY	Refer to Site Plan	1772739	5873842	38.1	150	-	144	237+	237+	198	1.62	47.0	1.10	2.70	8



SITE PLAN

Project No: 773-ETAM00829AA

Work Order No: ETAM18W04592

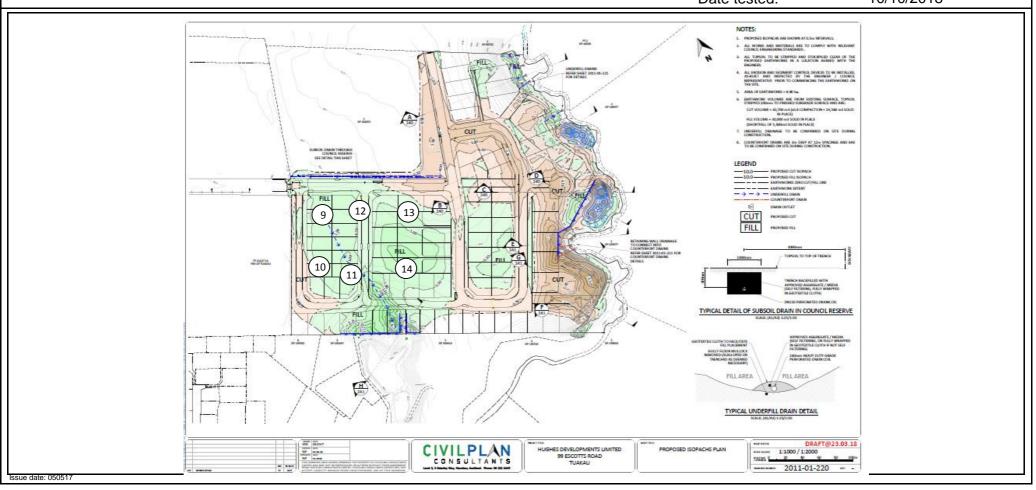
Page No: 2 of 2

NOT TO SCALE

Project: J00779 - 99 Escotts Road Tuakau

Location: As below Tested by: BS

Date tested: 10/10/2018





jes.

www.coffey.co

Client: Lander Geotechnical Consultants Limited

Address PO Box 97 385, Manukau 2241

Attention: Michael Chan

c.c:

Project: J00779 - 99 Escotts Road Tuakau

Location: Tuakau

PROJECT CODE: 773-ETAM00829AA

Page: 1 of 2

ACCREDITED LARGRATORY

Tests indicated as not accredited are outside the scope of the laboratory's accreditation

Approved Signatory:

Cesar Pura

Issue date: 31/10/2018

Test method: Test Methods in accordance with. *Shear Strength (using field Shear vane in accordance with NZGS 2001): Nuclear Densometer Testing (in accordance with NZS 4407:2015 Test 4.2): Water Content Testing (in accordance with NZS 4402:1986 Test 2.1): Moisture contents and dry densities are corrected against oven dried moisture content testing.

	and the definition of the control of																			
Date	Work Order No: ETAM	Tested by	Test No.	Layer	Material tested	Location	Easting	Northing	RL (m)	Probe Test Depth (mm)	Comments	Field Shear Strength in kPa				Wet Density (T/m ³)	Oven Water Content (%)	Dry Density (T/m3)	Solid Density (T/m3) Assumed	Air Voids (%)
11/10/20	18 ETAM18W04593	BS	15	Fill	Silty CLAY	Refer to Site Plan	1772676	5873933	-	150	Retest of Test No. 3	UTP	UTP	UTP	UTP	1.68	50.7	1.12	2.70	2
11/10/20	18 ETAM18W04593	BS	16	Fill	Silty CLAY	Refer to Site Plan	1772649	5873852	-	150	Retest of Test No. 5	UTP	UTP	UTP	UTP	1.60	51.9	1.05	2.70	7
11/10/20	18 ETAM18W04593	BS	17	Fill	Silty CLAY	Refer to Site Plan	172712	5873836	-	150	1.0 m below Finished Level	UTP	UTP	237+	237+	1.71	47.2	1.16	2.70	2
11/10/20	18 ETAM18W04593	BS	18	Fill	Silty CLAY	Refer to Site Plan	1772767	5873868	-	150	1.0 m below Finished Level	237+	225	198	233	1.60	62.6	0.99	2.70	2
11/10/20	18 ETAM18W04593	BS	19	Fill	Silty CLAY	Refer to Site Plan	1772741	5873822	-	150	1.0 m below Finished Level	237+	UTP	UTP	UTP	1.60	43.3	1.11	2.70	11
11/10/20	18 ETAM18W04593	BS	20	Fill	Silty CLAY	Refer to Site Plan	1772756	5873789	-	150	1.0 m below Finished Level	UTP	UTP	UTP	UTP	1.67	46.7	1.14	2.70	5



NOT TO SCALE

Project No: 773-ETAM00829AA

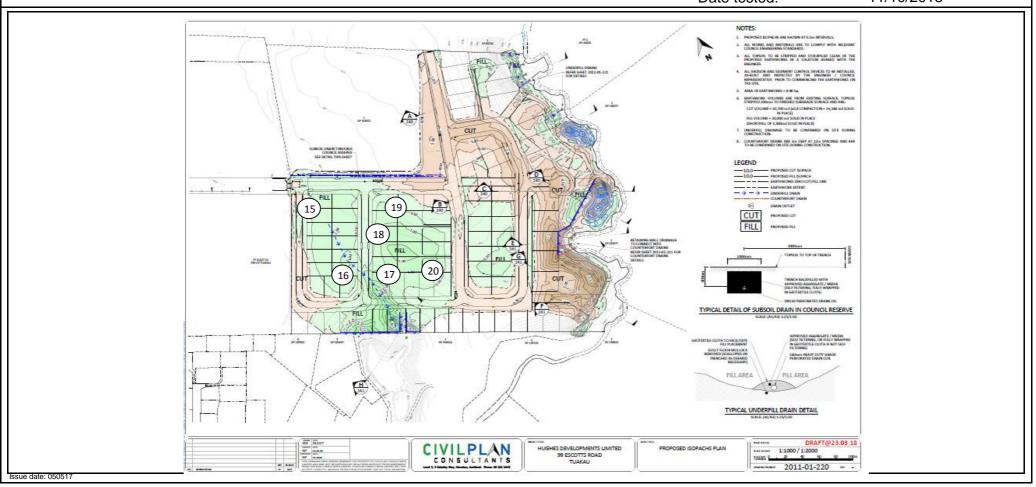
Work Order No: ETAM18W04593

Page No: 2 of 2

Project: J00779 - 99 Escotts Road Tuakau

Location: As below Tested by: BS

Date tested: 11/10/2018





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Client: Lander Geotechnical Consultants Limited

Address PO Box 97 385, Manukau 2241

Attention: Michael Chan

c.c:

Project: J00779 - 99 Escotts Road Tuakau

Location: Tuakau

PROJECT CODE: 773-ETAM00829AA

Page: 1 of 2

ACCREDITED LABORATORY

Tests indicated as not accredited are outside the scope of the laboratory's accreditation

Approved Signatory:

Cesar Pura

Issue date:

31/10/2018

jes.

Test method: Test Methods in accordance with NZS 4407:2015 Test 4.2): Water Content Testing (in accordance with NZS 4402:1986 Test 2.1): Moisture contents and dry densities are corrected against oven dried moisture content testing.

	and dry densities are	corrected a	gairist o	ven uneu	moisture contei	nictesting.														
Date	Work Order No: ETAM	Tested by	Test No.	Layer	Material tested	Location	Easting	Northing	RL (m)	Probe Test Depth (mm)	Comments	Field	d Shear S	trength in	ı kPa	Wet Density (T/m ³)	Oven Water Content (%)	Dry Density (T/m3)	Solid Density (T/m3) Assumed	Air Voids (%)
15/10/2018	ETAM18W04594	SC	21	Fill	Silty CLAY	Refer to Site Plan	1772741	5873822	-	150	Retest of Test No. 19	UTP	UTP	231+	231+	1.60	49.1	1.07	2.70	8
15/10/2018	ETAM18W04594	SC	22	Fill	Silty CLAY	Refer to Site Plan	1772768	5873871	-	150	0.5 m below Finished Level	231+	231+	UTP	UTP	1.63	60.1	1.02	2.70	1
15/10/2018	ETAM18W04594	SC	23	Fill	Silty CLAY	Refer to Site Plan	1772756	5873908	-	150	0.5 m below Finished Level	231+	231+	UTP	UTP	1.59	57.3	1.01	2.70	5
15/10/2018	ETAM18W04594	SC	24	Fill	Silty CLAY	Refer to Site Plan	1772732	5873880	-	150	0.5 m below Finished Level	UTP	UTP	231+	231+	1.55	56.7	0.99	2.70	7
15/10/2018	ETAM18W04594	SC	25	Fill	Silty CLAY	Refer to Site Plan	1772726	5878836	-	150	0.5 m below Finished Level	231+	231+	UTP	UTP	1.69	56.1	1.09	2.70	0



Project:

SITE PLAN

NOT TO SCALE

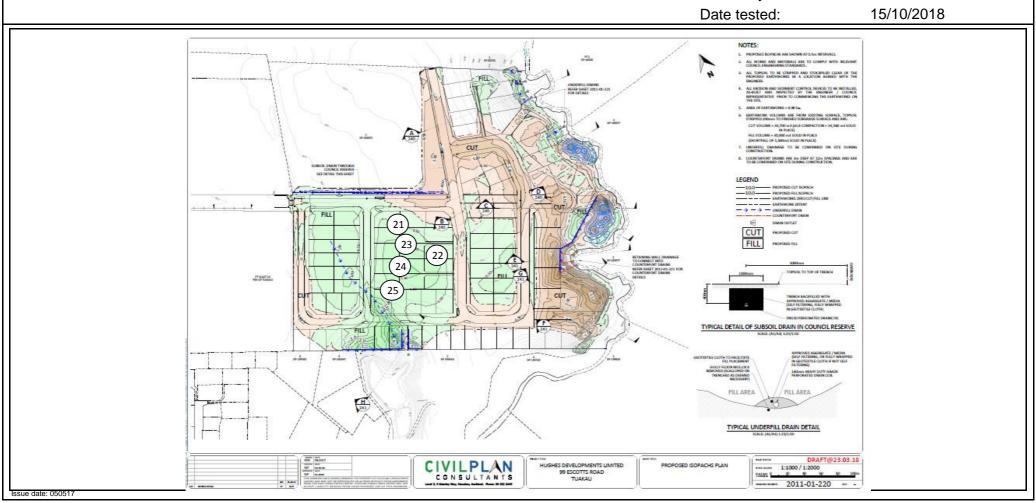
Project No: 773-ETAM00829AA

Work Order No: ETAM18W04595

Page No: 2 of 2

J00779 - 99 Escotts Road Tuakau

Location: As below Tested by: SC





jes.

Cesar Pura

www.coffey.co

Client: Lander Geotechnical Consultants Limited

Address PO Box 97 385, Manukau 2241

Attention: Michael Chan

c.c:

Project: J00779 - 99 Escotts Road Tuakau

Location: Tuakau

PROJECT CODE: 773-ETAM00829AA

Page: 1 of 2

ACCREDITED LABORATORY

Tests indicated as not accredited are outside the scope of the laboratory's accreditation

Approved Signatory:

. 04/40

Issue date: 31/10/2018

Test method: Test Methods in accordance with NZS 4407:2015 Test 4.2): Water Content Testing (in accordance with NZS 4402:1986 Test 2.1): Moisture contents and dry densities are corrected against oven dried moisture content testing.

Date	Work Order No: ETAM	Tested by	Test No.	Layer	Material tested	Location	Easting	Northing	RL (m)	Probe Test Depth (mm)	Comments	Field	d Shear S	trength in	kPa	Wet Density (T/m ³)	Oven Water Content (%)	Dry Density (T/m3)	Solid Density (T/m3) Assumed	Air Voids (%)
16/10/2018	ETAM18W04596	SC	26	Fill	Silty CLAY	Refer to Site Plan	1772760	5873880	•	150	0.3 m below Finished Level	231+	179	UTP	UTP	1.67	57.2	1.06	2.70	0
16/10/2018	ETAM18W04596	SC	27	Fill	Silty CLAY	Refer to Site Plan	1772735	5873836		150	0.3 m below Finished Level	206	231+	UTP	UTP	1.60	69.5	0.94	2.70	0
16/10/2018	ETAM18W04596	SC	28	Fill	Silty CLAY	Refer to Site Plan	1772763	5873820	-	150	0.3 m below Finished Level	231+	231+	231+	231+	1.65	59.5	1.03	2.70	0
16/10/2018	ETAM18W04596	SC	29	Fill	Silty CLAY	Refer to Site Plan	1772757	5873867	-	150	0.3 m below Finished Level	231+	231+	206	231+	1.65	73.7	0.95	2.70	0



NOT TO SCALE

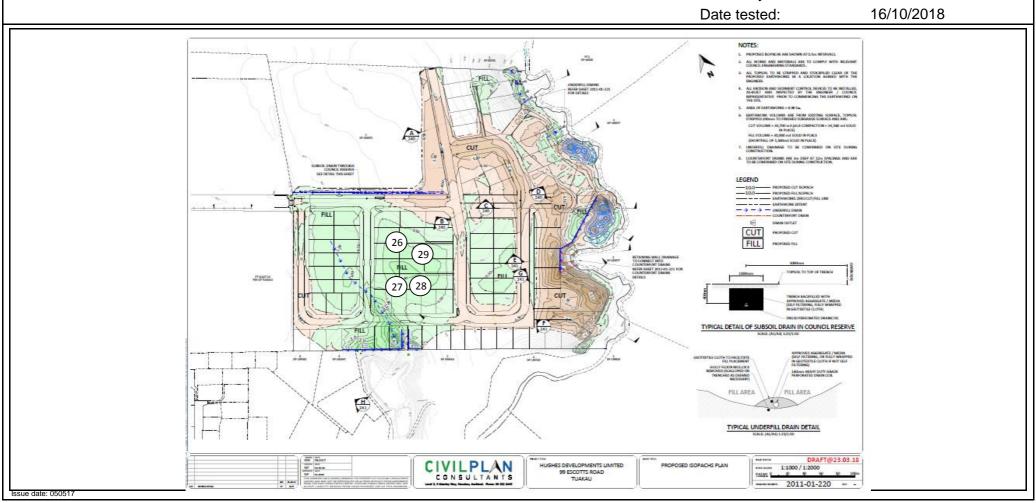
Project No: 773-ETAM00829AA

Work Order No: ETAM18W04596

Page No: 2 of 2

Project: J00779 - 99 Escotts Road Tuakau

Location: As below Tested by: SC





jes.

ww.coffey.con

Client: Lander Geotechnical Consultants Limited

Address PO Box 97 385, Manukau 2241

Attention: Michael Chan

c.c:

Location:

Project: J00779 - 99 Escotts Road Tuakau

Tuakau

PROJECT CODE:

1 of 2

773-ETAM00829AA

IANZ

Tests indicated as not accredited are outside the scope of the laboratory's accreditation

Approved Signatory: Cesar Pura

Issue date: 31/10/2018

Test method: Test Methods in accordance with NZS 4407:2015 Test 4.2): Water Content Testing (in accordance with NZS 4402:1986 Test 2.1): Moisture contents and dry densities are corrected against oven dried moisture content testing.

Page:

Date	Work Order No: ETAM	Tested by	Test No.	Layer	Material tested	Location	Easting	Northing	RL (m)	Probe Test Depth (mm)	Comments	Field	l Shear S	trength in	kPa		Oven Water Content (%)		Solid Density (T/m3) Assumed	Air Voids (%)
17/10/2018	ETAM18W04597	SC	30	Fill	Silty CLAY	Refer to Site Plan	1772773	5873941	-	150	-	231+	231+	231+	231+	1.67	44.1	1.16	2.7	6
17/10/2018	ETAM18W04597	SC	31	Fill	Silty CLAY	Refer to Site Plan	1772734	5873962	-	150	-	231+	231+	231+	231+	1.58	44.9	1.09	2.7	10



NOT TO SCALE

Project No: 773-ETAM00829AA

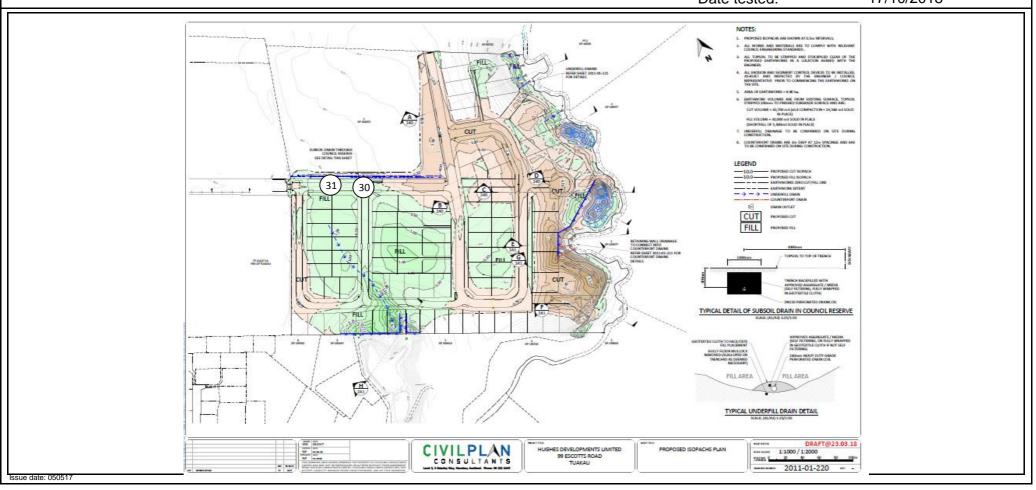
Work Order No: ETAM18W04597

Page No: 2 of 2

Project: J00779 - 99 Escotts Road Tuakau

Location: As below Tested by: SC

Date tested: 17/10/2018





jes.

Cesar Pura

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Client: Lander Geotechnical Consultants Limited

Address PO Box 97 385, Manukau 2241

Attention: Michael Chan

c.c:

Project: J00779 - 99 Escotts Road Tuakau

Location: Tuakau

PROJECT CODE: 773-ETAM00829AA

Page: 1 of 2

ACCREDITED LARGRATORY

Tests indicated as not accredited are outside the scope of the laboratory's accreditation

Approved Signatory:

Issue date: 31/10/2018

Test method: Test Methods in accordance with: *Shear Strength (using field Shear vane in accordance with NZGS 2001): Nuclear Densometer Testing (in accordance with NZS 4407:2015 Test 4.2): Water Content Testing (in accordance with NZS 4402:1986 Test 2.1): Moisture contents and dry densities are corrected against oven dried moisture content testing.

Date	Work Order No: ETAM	Tested by	Test No.	Layer	Material tested	Location	Easting	Northing	RL (m)	Probe Test Depth (mm)	Comments	Field	d Shear S	trength in	kPa	Wet Density (T/m ³)	Oven Water Content (%)	Dry Density (T/m3)	Solid Density (T/m3) Assumed	Air Voids (%)
19/10/2018	ETAM18W04598	SC	32	Fill	Silty CLAY	Refer to Site Plan	1772734	5873962	-	150	Retest of Test No.31	231+	231+	231+	231+	1.67	47.3	1.13	2.7	5
19/10/2018	ETAM18W04598	SC	33	Fill	Silty CLAY	Refer to Site Plan	1772738	5893845	-	150	At Finished Level	231+	231+	231+	231+	1.59	45.1	1.09	2.7	10
19/10/2018	ETAM18W04598	SC	34	Fill	Silty CLAY	Refer to Site Plan	1772752	5893874	-	150	At Finished Level	UTP	UTP	231+	231+	1.72	48.6	1.16	2.7	1
19/10/2018	ETAM18W04598	SC	35	Fill	Silty CLAY	Refer to Site Plan	1772756	5893898	,	150	At Finished Level	231+	231+	UTP	UTP	1.63	58.3	1.03	2.7	2



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Project No: 773-ETAM00829AA

Work Order No: E7

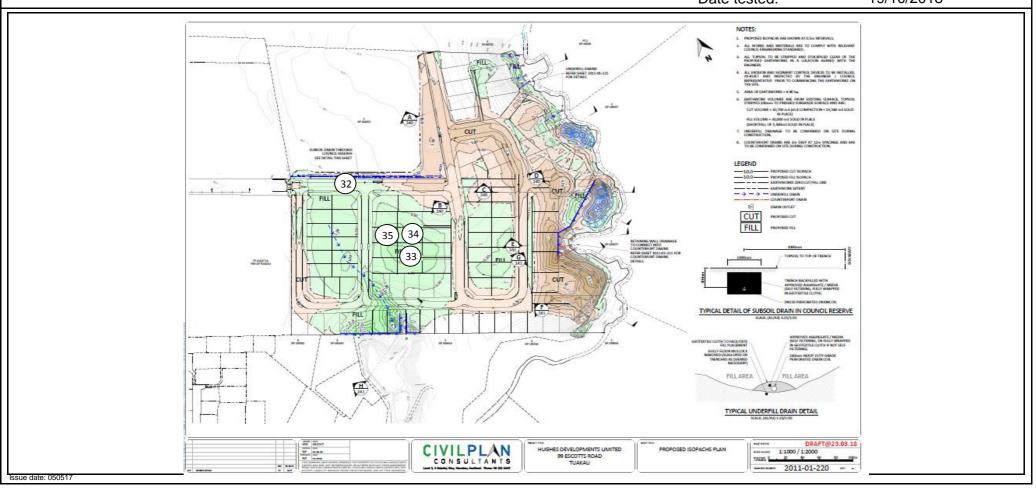
ETAM18W04598

Page No: 2 of 2

Project: J00779 - 99 Escotts Road Tuakau

Location: As below Tested by: SC

Date tested: 19/10/2018





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Client: Lander Geotechnical Consultants Limited

Address PO Box 97 385, Manukau 2241

Attention: Michael Chan

c.c:

Location:

Project: J00779 - 99 Escotts Road Tuakau

Tuakau

Dema

773-ETAM00829AA

Page: 1 of 2

ACCREDITED LARDRATORY

PROJECT CODE:

Tests indicated as not accredited are outside the scope of the laboratory's accreditation

Approved Signatory:

Cesar Pura

Issue date: 2/11/2018

Test method: Test Methods in accordance with NZS 4407:2015 Test 4.2): Water Content Testing (in accordance with NZS 4402:1986 Test 2.1): Moisture contents and dry densities are corrected against oven dried moisture content testing.

Date	Work Order No: ETAM	Tested by	Test No.	Layer	Material tested	Location	Easting	Northing	RL (m)	Probe Test Depth (mm)	Comments	Field	l Shear S	trength in	kPa		Oven Water Content (%)		Solid Density (T/m3) Assumed	Air Voids (%)
26/10/2018	ETAM18W04729	SC	36	Fill	Silty CLAY	Refer to Site Plan	1772738	5873845	-	150		UTP	UTP	231+	231+	1.69	40.9	1.20	2.7	7
26/10/2018	ETAM18W04729	SC	37	Fill	Silty CLAY	Refer to Site Plan	1772779	5873842	-	150		231+	231+	UTP	UTP	1.66	53.7	1.08	2.7	2



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Project No: 773-ETAM00829AA

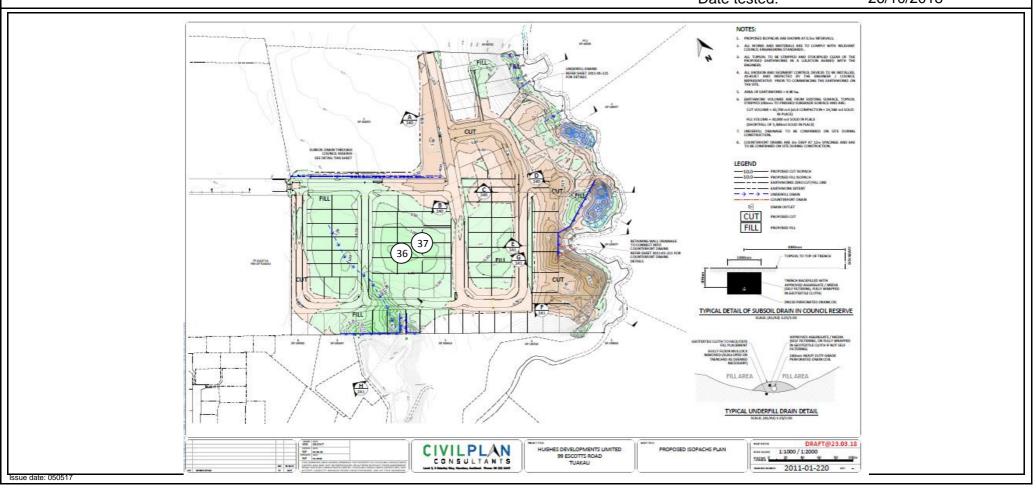
Work Order No: ETAM18W04729

Page No: 2 of 2

Project: J00779 - 99 Escotts Road Tuakau

Location: As below Tested by: SC

Date tested: 26/10/2018





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Cesar Pura

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Client: Lander Geotechnical Consultants Limited

Address PO Box 97 385, Manukau 2241

Attention: Michael Chan

c.c:

Project: J00779 - 99 Escotts Road Tuakau

Location: Tuakau

PROJECT CODE: 773-ETAM00829AA

Page: 1 of 2

ACCREDITED LARGRATORY

Tests indicated as not accredited are outside the scope of the laboratory's accreditation

Approved Signatory:

1/10/00

Issue date: 4/12/2018

Date	Work Order No: ETAM	Tested by	Test No.	Layer	Material tested	Location	Easting	Northing	RL (m)	Probe Test Depth (mm)	Comments	Field	d Shear S	trength in	kPa		Oven Water Content (%)	Dry Density (T/m3)	Solid Density (T/m3) Assumed	Air Voids (%)
28/11/2018	ETAM18W05320	SC	38	Fill	Silty CLAY	Retaining Wall Backfill	1772932	3893787	-	150	3m below Finished Level	231+	231+	231+	231+	1.66	49.5	1.11	2.7	4
28/11/2018	ETAM18W05320	SC	39	Fill	Silty CLAY	Retaining Wall Backfill	1772941	5893795	-	150	3m below Finished Level	231+	231+	231+	231+	1.62	51.6	1.07	2.7	5



Project:

SITE PLAN

Project No: 773-ETAM00829AA

Work Order No: E

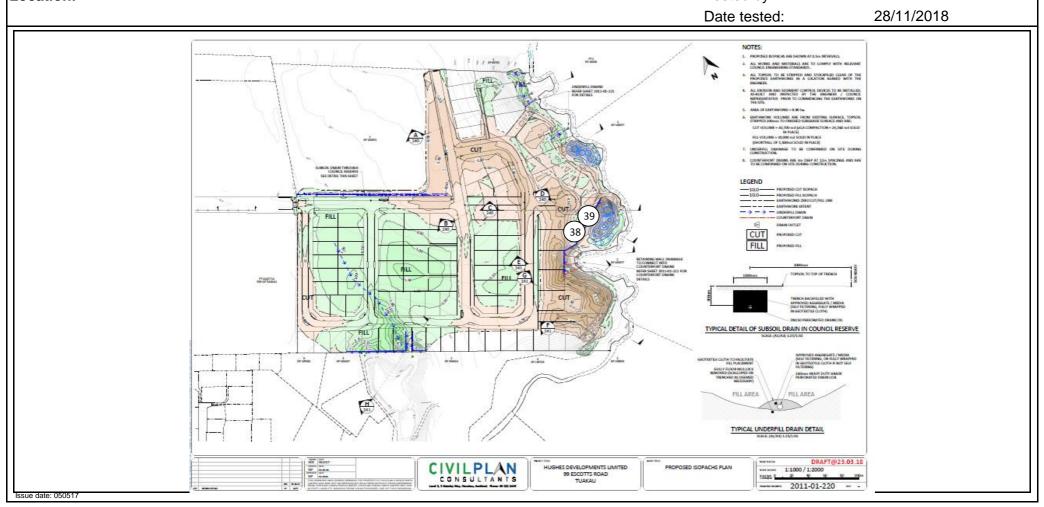
ETAM18W05320

Page No: 2 of 2

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J00779 - 99 Escotts Road Tuakau

Location: As below Tested by: SC





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Attention: Michael Chan

c.c:

Project: J00779 - 99 Escotts Road Tuakau

Location: Tuakau PROJECT CODE: 773-ETAM00829AA

Page: 1 of 2

Tests indicated as not accredited are outside the scope of the laboratory's accreditation

Approved Signatory:

Cesar Pura

Issue date: 4/12/2018

Test Methods in accordance with NZG 4407:2015 Test 4.2): Water Content Testing (in accordance with NZG 4402:1986 Test 2.1): Moisture contents Test method: and dry densities are corrected against oven dried moisture content testing.

Date	Work Order No: ETAM	Tested by	Test No.	Layer	Material tested	Location	Easting	Northing	RL (m)	Probe Test Depth (mm)		Field	l Shear S	trength in	kPa		Oven Water Content (%)	Dry Density (T/m3)	Solid Density (T/m3) Assumed	Air Voids (%)
29/11/2018	ETAM18W05326	SC	40	Fill	Silty CLAY	Retaining Wall Backfill	1772937	5873791	-	150	2m below Finished Level	206	206	191	191	1.64	48.3	1.11	2.7	5
29/11/2018	ETAM18W05326	SC	41	Fill	Silty CLAY	Retaining Wall Backfill	1772946	5873793	-	150	2m below Finished Level	206	206	191	191	1.70	48.7	1.15	2.7	2



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Project No: 773-ETAM00829AA

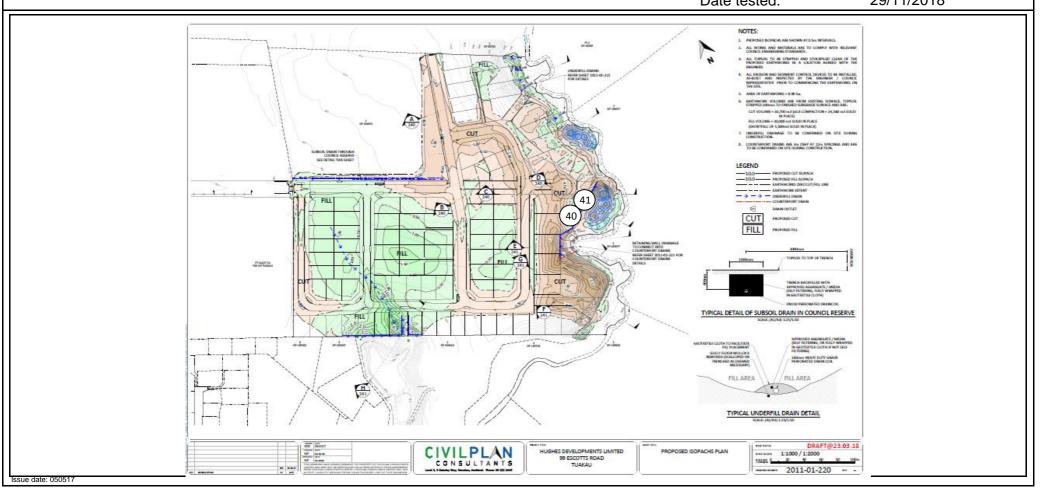
Work Order No: ETAM18W05326

Page No: 2 of 2

Project: J00779 - 99 Escotts Road Tuakau

Location: As below Tested by: SC

Date tested: 29/11/2018





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Address PO Box 97 385, Manukau 2241

Attention: Michael Chan

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Location:

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Page: 1 of 2

ACCREDITED LABORATORY

Tests indicated as not accredited are outside the scope of the laboratory's accreditation

Approved Signatory: Cesar Pura

Issue date: 5/12/2018

Test method:

Test Methods in accordance with NZS 4407:2015 Test 4.2): Water Content Testing (in accordance with NZS 4402:1986 Test 2.1): Moisture contents and dry densities are corrected against oven dried moisture content testing.

	and dry densities are	conecteu a	ayanısı u	ven unec	i illoisture conte	ant testing.														
Date	Work Order No: ETAM	Tested by	Test No.	Layer	Material tested	Location	Easting	Northing	RL (m)	Probe Test Depth (mm)	Comments	Field	l Shear S	trength in	kPa		Oven Water Content (%)		Solid Density (T/m3) Assumed	Air Voids (%)
29/11/2018	ETAM18W05352	SC	42	Fill	Silty CLAY	Retaining Wall Backfill	1772957	5873803	-	150	3m below Finished Level	191	191	231+	231+	1.71	45.8	1.17	2.7	3
29/11/2018	ETAM18W05352	SC	43	Fill	Silty CLAY	Retaining Wall Backfill	1772959	5873804	-	150	3m below Finished Level	206	206	206	206	1.77	46.3	1.21	2.7	0



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Project No: 773-ETAM00829AA

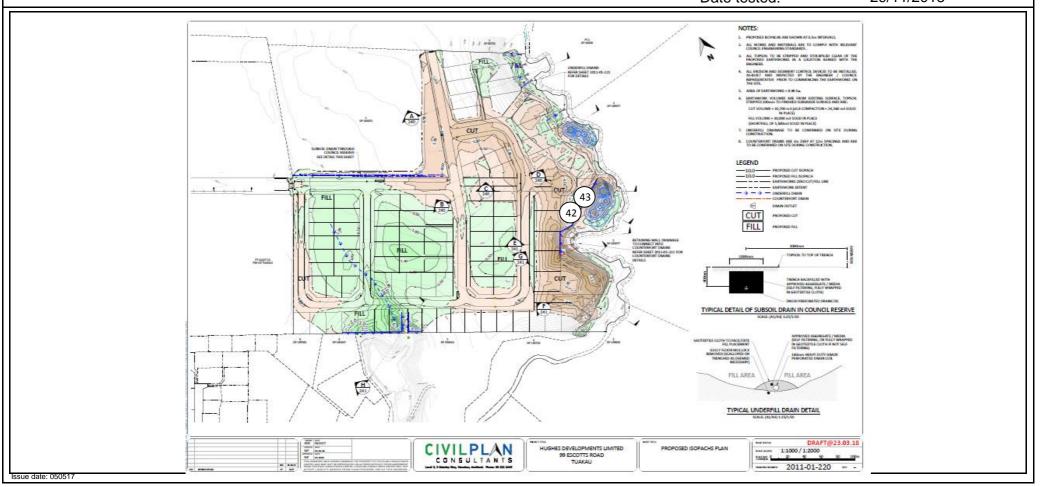
Work Order No: ETAM18W05352

Page No: 2 of 2

Project: J00779 - 99 Escotts Road Tuakau

Location: As below Tested by: SC

Date tested: 29/11/2018





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Address PO Box 97 385, Manukau 2241

Attention: Michael Chan

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Location:

Project: J00779 - 99 Escotts Road Tuakau

Tuakau

PROJECT CODE:

773-ETAM00829AA

Page: 1 of 2

LANZ
ACCPENITED LABORATORY

Tests indicated as not accredited are outside the scope of the laboratory's accreditation

Approved Signatory:

Cesar Pura

pes.

Issue date:

5/12/2018

Test method: Test method: Test Methods in accordance with NZS 4407:2015 Test 4.2): Water Content Testing (in accordance with NZS 4402:1986 Test 2.1): Moisture contents and dry densities are corrected against even died moisture contents.

	and dry densities are	conected a	iyaii ist u	wen uneu	moisture conte	ant testing.														
Date	Work Order No: ETAM	Tested by	Test No.	Layer	Material tested	Location	Easting	Northing	RL (m)	Probe Test Depth (mm)	Comments	Field	l Shear S	trength in	kPa	Wet Density (T/m ³)	Oven Water Content (%)		Solid Density (T/m3) Assumed	Air Voids (%)
30/11/2018	ETAM18W05369	SC	44	Fill	Silty CLAY	Retaining Wall Backfill	1772936	5873790	1	150	1m below Finished Level	206	206	206	206	1.71	49.3	1.15	2.7	1
30/11/2018	ETAM18W05369	SC	45	Fill	Silty CLAY	Retaining Wall Backfill	1772954	5873802	-	150	1m below Finished Level	191	170	170	170	1.68	51.6	1.11	2.7	2



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Project No: 773-ETAM00829AA

Work Order No:

ETAM18W05369

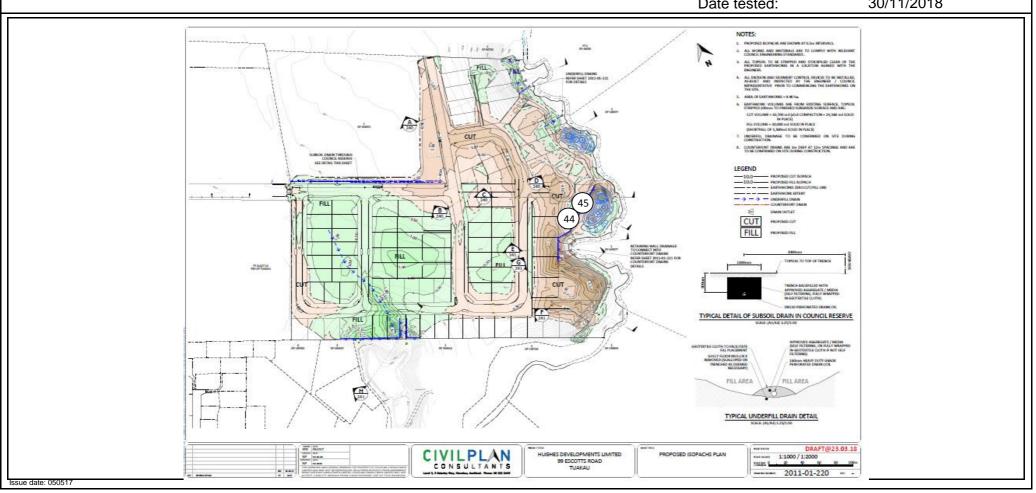
Page No:

2 of 2

J00779 - 99 Escotts Road Tuakau Project:

Tested by: SC Location: As below

> Date tested: 30/11/2018





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Client: Lander Geotechnical Consultants Limited

Address PO Box 97 385, Manukau 2241

Attention: Michael Chan

c.c:

Location:

Project: J00779 - 99 Escotts Road Tuakau

Tuakau

PROJECT CODE: 773-ETAM00829AA

Page: 1 of 2

ACCREDITED LABORATORY

Tests indicated as not accredited are outside the scope of the laboratory's accreditation

Approved Signatory: Cesar Pura

Issue date: 11/12/2018

Test method: Test Methods in accordance with NZS 4407:2015 Test 4.2): Water Content Testing (in accordance with NZS 4402:1986 Test 2.1): Moisture contents and dry densities are corrected against oven dried moisture content testing.

Date	Work Order No: ETAM	Tested by	Test No.	Layer	Material tested	Location	Easting	Northing	RL (m)	Probe Test Depth (mm)	Comments	Field	l Shear S	trength in	kPa	Wet Density (T/m³)	Oven Water Content (%)		Solid Density (T/m3) Assumed	Air Voids (%)
7/12/2018	ETAM18W05478	SC	46	Fill	Silty CLAY	Retaining Wall Fill	1772968	5873804	-	150	1.5m below Finished Level	119	124	114	124	1.74	47.6	1.18	2.7	0
7/12/2018	ETAM18W05478	SC	47	Fill	Silty CLAY	Retaining Wall Fill	1772973	5873811	-	150	1.5m below Finished Level	114	114	119	124	1.65	64.1	1.00	2.7	0



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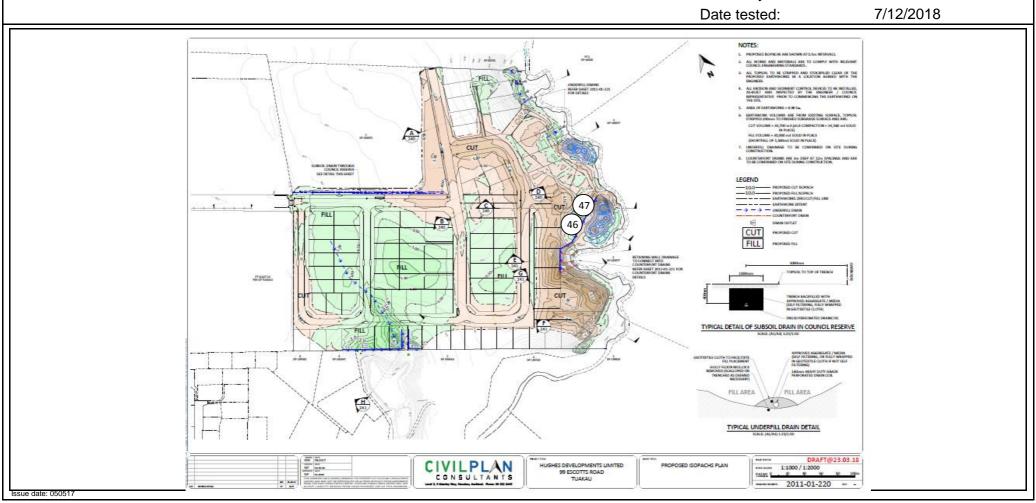
Project No: 773-ETAM00829AA

Work Order No: ETAM18W05478

Page No: 2 of 2

Project: J00779 - 99 Escotts Road Tuakau

Location: As below Tested by: SC





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Address PO Box 97 385, Manukau 2241

Attention: Michael Chan

c.c:

Location:

Project: J00779 - 99 Escotts Road Tuakau

Tuakau

PROJECT CODE: 773-ETAM00829AA

Page: 1 of 2

ACCREDITED LABORATORY

Tests indicated as not accredited are outside the scope of the laboratory's accreditation

Approved Signatory: Cesar Pura

Issue date: 11/12/2018

Test method:

Test Methods in accordance with NZS 4407:2015 Test 4.2): Water Content Testing (in accordance with NZS 4402:1986 Test 2.1): Moisture contents and dry densities are corrected against oven dried moisture content testing.

	and dry densities are	e confected a	iyaii ist u	wen uneu	moisture conte	ant testing.														
Date	Work Order No: ETAM	Tested by	Test No.	Layer	Material tested	Location	Easting	Northing	RL (m)	Probe Test Depth (mm)	Comments	Field	d Shear S	trength in	kPa	Wet Density (T/m³)	Oven Water Content (%)		Solid Density (T/m3) Assumed	Air Voids (%)
7/12/2018	ETAM18W05480	SC	48	Fill	Silty CLAY	Retaining Wall Fill	1772968	5873804	-	150	Retest of Test No. 46	231+	231+	231+	231+	1.71	49.2	1.14	2.7	1
7/12/2018	ETAM18W05480	SC	49	Fill	Silty CLAY	Retaining Wall Fill	1772973	5873811	-	150	Retest of Test No. 47	231+	231+	231+	231+	1.67	49.8	1.12	2.7	3



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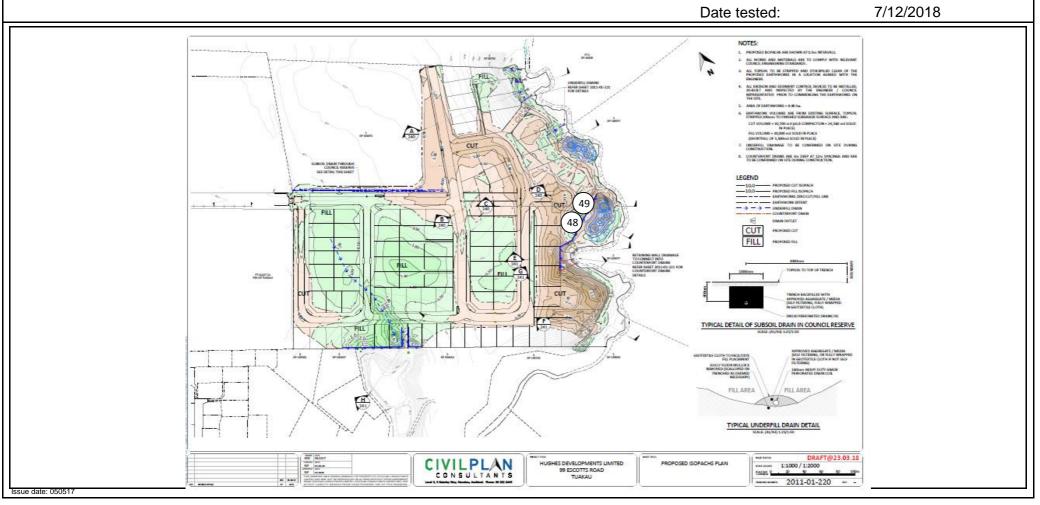
Project No: 773-ETAM00829AA

Work Order No: ETAM18W05480

Page No: 2 of 2

Project: J00779 - 99 Escotts Road Tuakau

Location: As below Tested by: SC





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Address PO Box 97 385, Manukau 2241

Attention: Michael Chan

c.c:

Location:

Project: J00779 - 99 Escotts Road Tuakau

Tuakau

PROJECT CODE: 773-ETAM00829AA

Page: 1 of 2

ACCREDITED LABORATORY

Tests indicated as not accredited are outside the scope of the laboratory's accreditation

Approved Signatory: Cesar Pura

Issue date: 12/12/2018

Test method: Test Methods in accordance with: *Shear Strength (using field Shear vane in accordance with NZGS 2001): Nuclear Densometer Testing (in accordance with NZS 4407:2015 Test 4.2): Water Content Testing (in accordance with NZS 4402:1986 Test 2.1): Moisture contents and dry densities are corrected against oven died moisture contents.

	and ary denomics are	, 001100104	igainot o	von anca	i inioiotaro come	nit tooting.														
Date	Work Order No: ETAM	Tested by	Test No.	Layer	Material tested	Location	Easting	Northing	RL (m)	Probe Test Depth (mm)	Comments	Field	l Shear S	trength in	kPa		Oven Water Content (%)		Solid Density (T/m3) Assumed	Air Voids (%)
10/12/2018	ETAM18W05504	SC	50	Fill	Silty CLAY	Retaining Wall Fill	1772944	5873792	-	150	At Finished Level	231+	231+	231+	231+	1.64	48.8	1.10	2.7	6
10/12/2018	ETAM18W05504	SC	51	Fill	Silty CLAY	Retaining Wall Fill	1772959	5873806	-	150	At Finished Level	231+	231+	231+	231+	1.64	54.0	1.07	2.7	3



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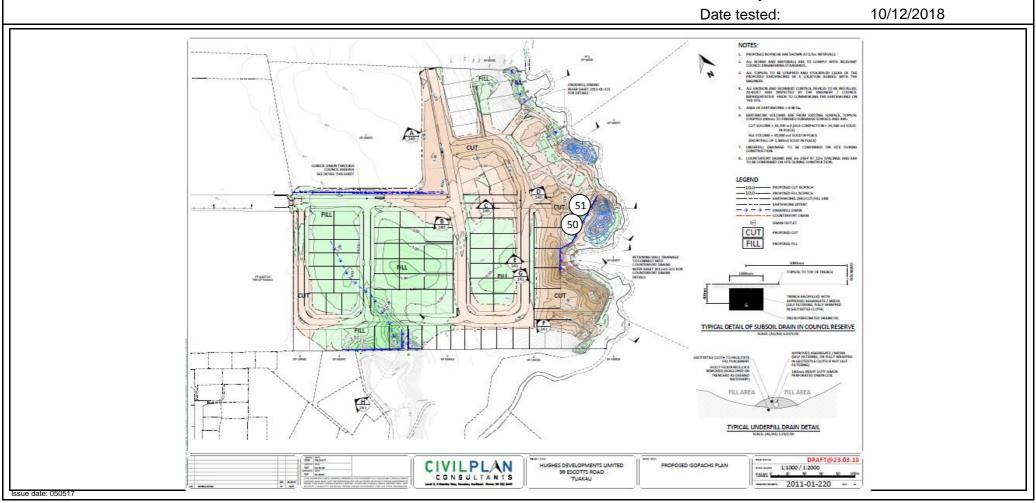
Project No: 773-ETAM00829AA

Work Order No: ETAM18W05504

Page No: 2 of 2

Project: J00779 - 99 Escotts Road Tuakau

Location: As below Tested by: SC





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Client: Lander Geotechnical Consultants Limited

Address PO Box 97 385, Manukau 2241

Attention: Michael Chan

c.c:

Location:

Project: J00779 - 99 Escotts Road Tuakau

Tuakau

PROJECT CODE: 773-ETAM00829AA

Page: 1 of 2

ACCREDITED LABORATORY

Tests indicated as not accredited are outside the scope of the laboratory's accreditation

Approved Signatory: Cesar Pura

Issue date: 14/12/2018

Test method: Test Methods in accordance with NZS 4407:2015 Test 4.2): Water Content Testing (in accordance with NZS 4402:1986 Test 2.1): Moisture contents and dry densities are corrected against oven dried moisture content testing.

Date	Work Order No: ETAM	Tested by	Test No.	Layer	Material tested	Location	Easting	Northing	RL (m)	Probe Test Depth (mm)	Comments	Field Shear Strength in kPa			kPa		Oven Water Content (%)	Dry Density (T/m3)	Solid Density (T/m³) Assumed	Air Voids (%)
11/12/2018	ETAM18W05592	SC	52	Fill	Silty CLAY	Retaining Wall Fill	1772928	5873784	-	150	1.0m below Finished Level	231+	231+	231+	231+	1.76	43.4	1.23	2.70	1
11/12/2018	ETAM18W05592	SC	53	Fill	Silty CLAY	Retaining Wall Fill	1772920	5873786	-	150	1.0m below Finished Level	231+	231+	231+	231+	1.75	40.5	1.24	2.70	3



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Project No: 773-ETAM00829AA

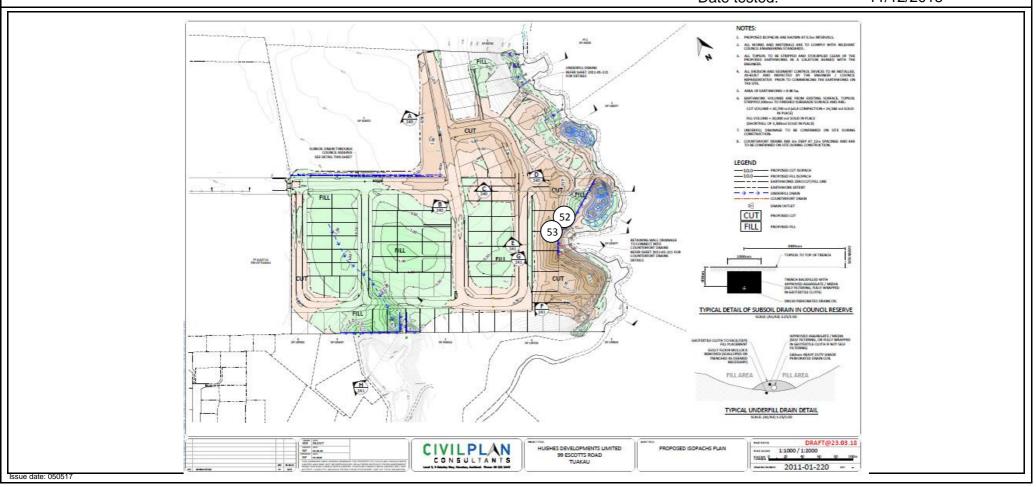
Work Order No: ETAM18W05592

Page No: 2 of 2

Project: J00779 - 99 Escotts Road Tuakau

Location: As below Tested by: SC

Date tested: 11/12/2018





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Cesar Pura

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Client: Lander Geotechnical Consultants Limited

Address PO Box 97 385, Manukau 2241

Attention: Michael Chan

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Location:

Project: J00779 - 99 Escotts Road Tuakau

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PROJECT CODE: 773-ETAM00829AA

Page: 1 of 2

ACCREDITED LABORATORY

Tests indicated as not accredited are outside the scope of the laboratory's accreditation

Approved Signatory:

Issue date: 17/12/2018

Test method: Test Methods in accordance with: *Shear Strength (using field Shear vane in accordance with NZGS 2001): Nuclear Densometer Testing (in accordance with NZS 4407:2015 Test 4.2): Water Content Testing (in accordance with NZS 4402:1986 Test 2.1): Moisture contents and dry densities are corrected against oven died moisture contents.

Date	Work Order No: ETAM	Tested by	Test No.	Layer	Material tested	Location	Easting	Northing	RL (m)	Probe Test Depth (mm)	Comments	Field Shear Strength in kPa					Oven Water Content (%)		Solid Density (T/m³) Assumed	Air Voids (%)
12/12/2018	ETAM18W05606	SC	54	Fill	Silty CLAY	Retaining Wall Fill	1772919	5873787	-	150	At Finished Level	231+	231+	231+	231+	1.74	43.8	1.21	2.70	2
12/12/2018	ETAM18W05606	SC	55	Fill	Silty CLAY	Retaining Wall Fill	1772927	5873786	-	150	At Finished Level	231+	231+	231+	231+	1.70	40.0	1.21	2.70	7



Project No: 773-ETAM00829AA

Work Order No: ETAM18W05606

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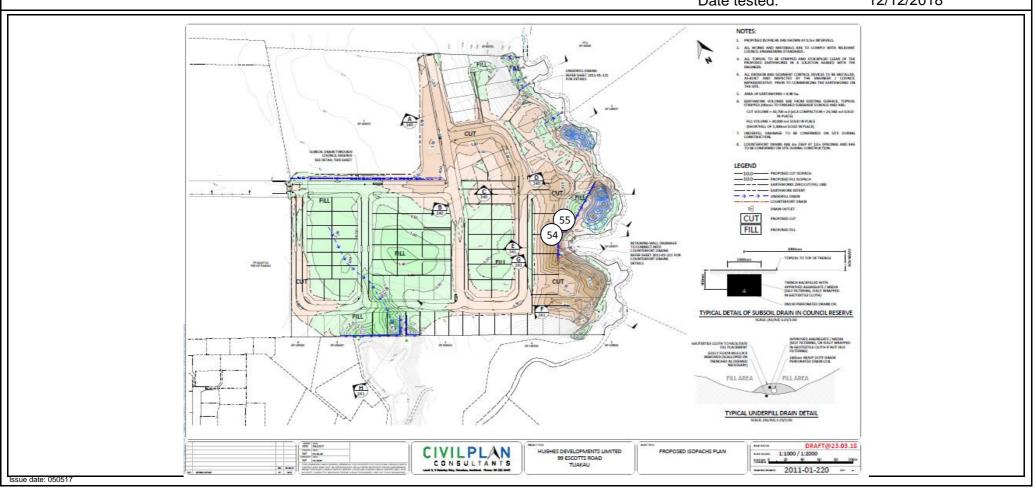
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Page No: 2 of 2

Project: J00779 - 99 Escotts Road Tuakau

Location: As below Tested by: SC

Date tested: 12/12/2018





Client: Lander Geotechnical Consultants Limited

Address PO Box 97 385, Manukau 2241

Attention: Michael Chan

Tuakau

c.c:

Location:

Project: J00779 - 99 Escotts Road Tuakau PROJECT CODE:

Page: 1 of 2

Tests indicated as not accredited are outside the scope of the laboratory's accreditation

773-ETAM00829AA

pes. Cesar Pura

Approved Signatory:

Issue date: 18/12/2018

Test Methods in accordance with NZG 4407:2015 Test 4.2): Water Content Testing (in accordance with NZG 4402:1986 Test 2.1): Moisture contents Test method:

Date	Work Order No: ETAM	Tested by	Test No.	Layer	Material tested	Location	Easting	Northing	RL (m)	Probe Test Depth (mm)	Comments	Field Shear Strength in kPa					Oven Water Content (%)	Dry Density (T/m3)	Solid Density (T/m³) Assumed	Air Voids (%)
14/12/2018	ETAM18W05627	SC	56	Fill	Silty CLAY	General Fill	1772645	5873820	-	150	At Finished Level	UTP	UTP	231+	231+	1.65	44.9	1.14	2.70	7
14/12/2018	ETAM18W05627	SC	57	Fill	Silty CLAY	General Fill	1772642	5873808	-	150	At Finished Level	UTP	UTP	231+	231+	1.61	49.4	1.08	2.70	7



NOT TO SCALE

Project No: 773-ETAM00829AA

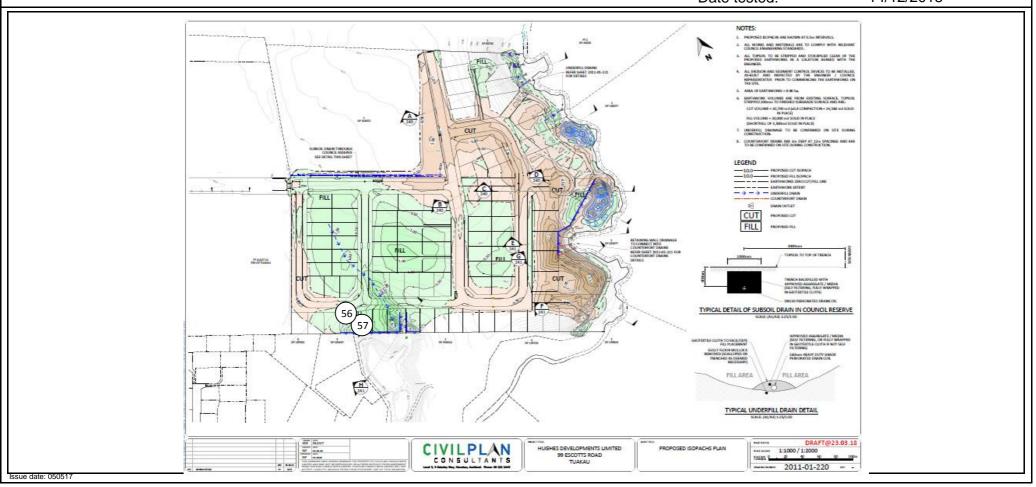
Work Order No: ETAM18W05627

Page No: 2 of 2

Project: J00779 - 99 Escotts Road Tuakau

Location: As below Tested by: SC

Date tested: 14/12/2018





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Address PO Box 97 385, Manukau 2241

Attention: Michael Chan

Tuakau

c.c:

Location:

Project: J00779 - 99 Escotts Road Tuakau

PROJECT CODE: 773-ETAM00829AA

Page: 1 of 2

ACCREDITED LABORATORY

Tests indicated as not accredited are outside the scope of the laboratory's accreditation

pel

Approved Signatory:

Issue date:

7/01/2019

Test method: Test Methods in accordance with: *Shear Strength (using field Shear vane in accordance with NZGS 2001): Nuclear Densometer Testing (in accordance with NZS 4407:2015 Test 4.2): Water Content Testing (in accordance with NZS 4402:1986 Test 2.1): Moisture contents and dry densities are corrected against oven dried moisture content testing.

Date	Work Order No: ETAM	Tested by	Test No.	Layer	Material tested	Location	Easting	Northing	RL (m)	Probe Test Depth (mm)	Comments	Field Shear Strength in kPa			kPa		Oven Water Content (%)	Dry Density (T/m3)	Solid Density (T/m³) Assumed	Air Voids (%)
18/12/2018	ETAM18W05761	SC	58	Fill	Silty CLAY	Refer to Site Plan	1772689	5873801	-	150	1.5m below Finished Level	206	206	231+	231+	1.61	49.8	1.07	2.70	7
18/12/2018	ETAM18W05761	SC	59	Fill	Silty CLAY	Refer to Site Plan	1772848	5873791	-	150	1.5m below Finished Level	231+	231+	231+	231+	1.85	33.9	1.38	2.70	2
18/12/2018	ETAM18W05761	SC	60	Fill	Silty CLAY	Refer to Site Plan	1772847	5873832	-	150	1.5m below Finished Level	231+	231+	231+	231+	1.86	40.1	1.33	2.70	0
18/12/2018	ETAM18W05761	SC	61	Fill	Silty CLAY	Refer to Site Plan	1772812	5873803	-	150	1.5m below Finished Level	231+	231+	231+	231+	1.74	47.2	1.19	2.70	0



NOT TO SCALE

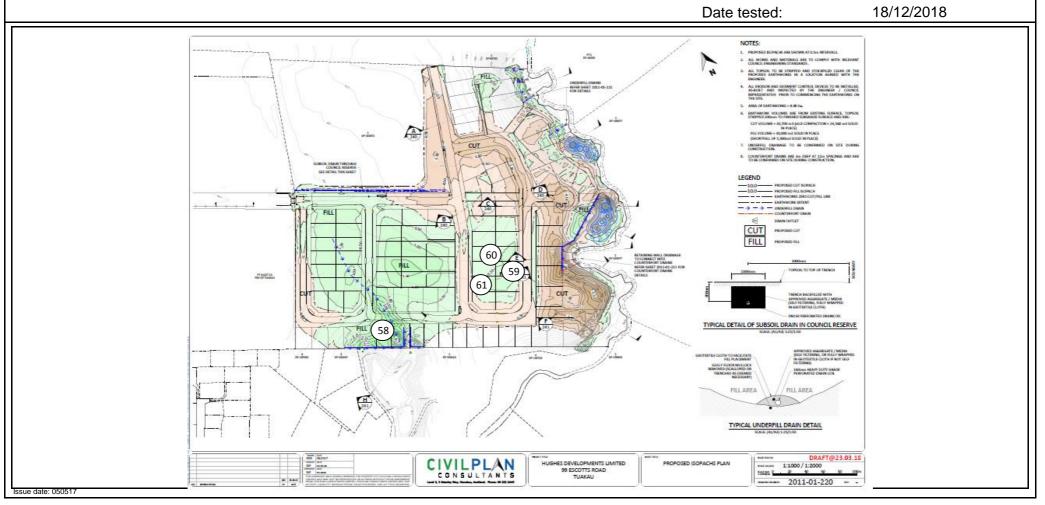
Project No: 773-ETAM00829AA

Work Order No: ETAM18W05761

Page No: 2 of 2

Project: J00779 - 99 Escotts Road Tuakau

Location: As below Tested by: SC



Our Ref - 1009213.0070.0.0/1 Page 1 of 25



Our Ref: 1009213.0070.0.0/1 Customer Ref: J00779

5 June 2019

Lander Geotechnical Ltd PO Box 97 385 Manukau Auckland 2241

Attention: Shane Lander

Dear Shane

99 Escotts Road, Tuakau Site Report

Customer's Instructions

We were instructed to:

Complete nuclear densometer, shear vane and impact hammer testing at the above mentioned site when requested and report the results.

Specifications

As per email from Mike (Lander Geotechnical) on the 8/01/2019 the specification for Earthworks Testing was as follows;

- Average air voids <10% with maximum air voids 12%.
- Average shear strength >140 kPa with minimum single value 120 kPa.
- Average value to be determined over 10 consecutive tests.

Dates of Procedures

Testing was carried out from the 16/1/2019 to the 8/05/2019.

Locations

Testing was carried out as instructed by the contractor on site. Test locations were selected on site by the Geotechnics technician on behalf of the customer.

The attached plan provides indicative locations only and is not to scale. All other information we provide regarding location should be referenced to the asset owner.

Samples

Samples taken for moisture content verification purposes were disposed of 24 hours after testing.

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Methods

NZGS 8:2001 - Test method for determining the vane shear strength of a cohesive soil using a hand held shear vane.

NZS 4407:1991 Test 4.2.1 - Method using a nuclear surface moisture-density gauge (Direct Transmission Mode) – NDM

NZS 4402:1986 Test 2.1 - Determination of water content

ASTM D 5874-16 - Standard test method for determination of the impact value (IV) of a soil – Impact Test

Material Description

Material descriptions are provided in the attached results.

Results

The following is attached:

Earthworks testing results, impact hammer results and testing location plans.

Test Remarks

Shear Vane

Shear Vane tests are potentially unsuitable for material described in the earthworks summary as 'Clay SILT w gravels' or 'Clay Strippings'. Tests in these materials may not be compliant with the stated test method and results are therefore not covered under the IANZ endorsement of this report. Results are provided for your own interpretation and inference.

NDM – Direct Transmission

The test method may not be appropriate for materials containing a nominal maximum particle size of >40 mm.

Nuclear densometers are calibrated for a bulk density range of 1,728 kg/m³ to 2,756 kg/m³. Test results outside of these bulk density limits are not covered under the IANZ endorsement of this report.

An assumed solid density value of 2.70t/m³ was agreed with the customer. We do not take responsibility for misrepresentation or misinterpretation arising from the use of this assumed value to calculate air voids.

Oven calculated air voids (%) have been reported as zero if negative. The calculation of air voids is based on wet density, moisture content and the solid density. The wet density is measured by the nuclear densometer and the moisture content by oven drying. The calculation of air voids is not part of the test in NZS 4407 and is therefore not covered under the IANZ endorsement of this report.

Determination of Water Content

Samples used for the determination of the water content were taken in conjunction with nuclear densometer testing and disposed of after 24 hours.

Impact Value

Field procedure A was used. The test method is appropriate for materials having a maximum particle size of less than 37.5 mm. Results of tests that were performed on materials containing larger particle sizes, are not covered under the IANZ endorsement of this report.

Calculations of equivalent CBR are based on CBR = $IV^2x0.07$.

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Pass/Fail Criteria

We accept no liability for any circumstances that may arise due to the inclusion of the pass/fail criteria or the use of this information by third parties. Pass/fail criteria are based solely on numerical values with no consideration given to uncertainty and are not covered under the IANZ endorsement of these results.

General Remarks

This report has been prepared for the benefit of Lander Geotechnical Ltd, with respect to the particular brief given to us and it cannot be relied upon in other contexts or for any other purpose without our prior review and agreement. The inherent uncertainties of site investigation work, mean the nature and continuity of subsoil away from the test location could vary from the data logged.

We provide the results for your interpretation and inference.

Descriptions are enclosed for your information, but are not covered under the IANZ endorsement of this report.

Sample(s) not destroyed during testing will be retained for one month from the date of this report before being discarded.

Please reproduce this report in full when transmitting to others or including in internal reports.

If we can be of any further assistance, feel free to get in touch. Contact details are provided at the bottom of the letterhead page.

GEOTECHNICS LTD

Report prepared by: Authorised for Geotechnics by:

Lack Aller

Josh Allan Steven Anderson
Project Manager Project Director
Approved Signatory

Report checked by:

Tests indicated as not accredited are outside the scope of the laboratory's accreditation

Daniel Brasting

CMT Field Coordinator

5-Jun-19

t:\geotechnicsgroup\projects\1009213\1009213.0070 - 99 escotts road, tuakau\workingmaterial\20190605 jra rpt1.docx

Our Ref - 1009213.0070.0.0/1 Page 4 of 25



Job: 99 Escotts Road, Tuakau

Client: Lander Geotechnical Ltd.

Job#	1009213.0070.0.0/1
Entered By	JRA
Checked By	VIVE / SEBA
Approved By	SIA

						Test Type		NDM 0 ⁰			NDM 90 ⁰		A	VERAGE ND	М	Solid	_	Final Co	rrected			s	hear Vane	Reading (kPa)			PASS / FAIL
URN	Tech.	Date	Location	Layer	Material	NDM / SV	Wet Density	Moisture Content	Air Voids		Moisture Content	Air Voids	Wet	Moisture Content	Air Voids	Density (t/m³)	Oven Moisture content (%)	Oven Dry Density	Average Air Voids	Average Air Voids (10 X Tests)	Reading	Reading	Reading	Reading	Average SV (4 x	Average	Retest URN	(P) Pass Comments
						NDIVI / SV	(t/m³)	(%)	(%)	(t/m³)	(%)	(%)	(t/m³)	(%)	(%)	Assumed		(t/m³)	(%)	(,	1	2	3	4	Tests)	(10 X Sets)		(F) Fail
1.1	JRA	16/01/2019	Retaining Wall	~1m below FL	Clay SILT	NDM / SV	1.63	41.9	9.1	1.66	39.6	8.7	1.65	40.8	8.9	2.70	52.3	1.08	3.3	-	154	136	121	124	134	-	-	P
1.2			Backfill			NDM / SV	1.63	48.7	6.2	1.63	48.0	6.4	1.63	48.4	6.3	2.70	51.6	1.07	4.8	-	184	163	136	199	171	-	-	P
2.1	JRA	24/01/2019	North East Fill - See Site Plan	RL 29.8	Clay SILT w gravels	NDM / SV	1.79	37.3	3.1	1.78	38.0	3.2	1.78	37.7	3.2	2.70	43.2	1.25	0.0	-	187	211	166	211	194	•	-	P
2.2			South Retaining	RL 29.7		NDM / SV	1.81	35.9	3.1	1.80	36.7	3.0	1.80	36.3	3.0	2.70	41.0	1.28	0.3	-	211	211	193	208	206	-	-	P
3.1	IRA	30/01/2019	Wall Fill - See Site	RL 30.4	Clay SILT	NDM / SV	1.54	51.6	9.8	1.54	52.2	9.7	1.54	51.9	9.7	2.70	51.9	1.01	9.7	-	166	130	157	181	159	-	-	P
3.2	JIM	30/01/2019	South East Fill - See Site Plan	RL 37.6	Clay SILI	NDM / SV	1.60	44.8	9.6	1.60	42.6	10.9	1.60	43.7	10.2	2.70	41.2	1.13	11.5	-	211	211	UTP	UTP	211		-	P
4.1	JRA	1/02/2019	South Retaining Wall Fill - See Site	~FL	Clay SILT	NDM / SV	1.59	58.6	4.3	1.59	59.2	3.8	1.59	58.9	4.1	2.70	67.0	0.95	1.0	-	195	196	169	211	193	-	-	P
4.2	JIVA	1/02/2019	Plan	r.	Clay SILI	NDM / SV	1.57	52.3	8.0	1.57	52.0	8.3	1.57	52.2	8.2	2.70	50.7	1.04	8.8	-	178	163	196	166	176	-	-	P
5.1	JRA	9/02/2019	South Retaining Wall Fill - See Site	~1m below FL	Clay SILT	NDM / SV	1.63	51.6	4.8	1.63	51.9	4.8	1.63	51.8	4.8	2.70	51.9	1.07	4.7	-	175	208	211	196	198	-		P Retests after fill from URN 3 & 4 was removed and replaced
5.2	3104	3/02/2013	Plan	III DEIOW I E	Clay SIL1	NDM / SV	1.66	51.9	2.9	1.66	49.9	3.6	1.66	50.9	3.2	2.70	48.5	1.12	4.3	4.8	199	211	211	193	204	184	-	P
6.1	JRA	2/03/2019	South Retaining Wall Fill	~2m below FL	Clay SILT	NDM / SV	1.68	40.9	7.1	1.67	40.2	7.7	1.68	40.6	7.4	2.70	36.2	1.23	9.8	5.5	185	185	UTP	185	185	189	-	P
6.2		2,10,100	North East Fill	~FL	City Sici	NDM / SV	1.73	39.8	5.1	1.72	41.6	4.4	1.72	40.7	4.7	2.70	39.0	1.24	5.7	5.6	185	185	185	UTP	185	191	-	P
7.1	JRA	8/03/2019	South Retaining	~1m below FL	Clay SILT	NDM / SV	1.66	42.2	7.6	1.66	39.2	9.3	1.66	40.7	8.5	2.70	39.6	1.19	9.0	6.5	185	UTP	UTP	182	184	190	-	P
7.2			Wall Fill			NDM / SV	1.80	38.1	1.9	1.80	37.0	2.6	1.80	37.6	2.3	2.70	34.6	1.34	4.1	6.9	185	185	185	164	180	187	-	P
8.1	JRA	13/03/2019	South Retaining	~FL	Clay SILT	NDM / SV	1.72	50.5	0.1	1.72	48.8	0.8	1.72	49.7	0.4	2.70	44.9	1.19	2.8	6.2	185	185	185	185	185	190	-	P
8.2			Wall Fill			NDM / SV	1.73	45.0	2.1	1.73	45.1	2.2	1.73	45.1	2.1	2.70	41.5	1.22	4.0	5.4	185	185	185	185	185	187	-	P
9.1	JRA	14/03/2019	South Retaining Wall Fill	Base of RW	Clay SILT	NDM / SV	1.66	50.0	3.5	1.65	50.3	4.0	1.66	50.2	3.8	2.70	51.4	1.09	3.2	5.6	185	142	185	145	164	184	-	P
9.2			Wall Fill	Toe of Batter		NDM / SV	1.68	47.7	3.5	1.68	48.9	3.2	1.68	48.3	3.4	2.70	46.7	1.14	4.1	5.2	185	185	161	148	170	184	-	P
10.1	JRA	19/03/2019	North East Fill - See Site Plan	~0.3m below FL	Clay SILT	NDM / SV	1.79	36.2	3.8	1.79	37.5	3.0	1.79	36.9	3.4	2.70	35.8	1.32	4.0	5.1	185	185	148	185	176	182	-	P
10.2			See Site Fiail			NDM / SV	1.71	41.3	5.2	1.71	41.7	5.0	1.71	41.5	5.1	2.70	38.1	1.24	7.0	5.4	185	185	185	185	185	180	-	P
11.1	VIVE	27/03/2019	North East Fill - See Site Plan	~1m placed	Clay SILT	NDM / SV	1.79	34.1	5.1	1.77	34.8	5.8	1.78	34.5	5.5	2.70	48.1	1.20	0.0	4.4	160	211	196	211	195	181	-	P
11.2			See Site Flair	~1.5m below top		NDM / SV		33.2	18.4	1.54	31.9	19.3	1.55	32.6	18.8	2.70	36.1	1.14	16.9	-	178	163	211	205	189	-	20.2	F
12.1	JRA	5/04/2019	South Retaining Wall Fill	of RW	Clay SILT w gravels	NDM / SV	1.84	36.8	0.7	1.84	36.7	0.9	1.84	36.8	0.8	2.70	38.5	1.33	0.0	4.6	185	185	185	185	185	181	-	P
12.2			wall Fill	~1m below top of RW		NDM / SV	1.92	30.5	0.9	1.88	32.6	1.2	1.90	31.6	1.0	2.70	34.1	1.42	0.0	4.2	185	185	185	185	185	182	-	P
13.1	JRA	9/04/2019	Old SILT Pond & South Retaining	~1.5m below top of RW	Clay SILT w gravels	NDM / SV	1.77	41.9	1.6	1.76	40.9	2.5	1.77	41.4	2.1	2.70	40.7	1.25	2.5	4.2	185	161	185	171	176	181	-	P
13.2		.,.,	Wall Fill	~2m below top of RW	, 8	NDM / SV	1.78	39.2	2.7	1.78	40.1	1.9	1.78	39.7	2.3	2.70	40.1	1.27	2.0	4.0	148	185	174	145	163	179	-	P
15.1	VIVE	16/04/2019	Fill Area-See Site	~0.7m Placed	Clay SILT w gravels	NDM / SV	1.71	38.2	6.9	1.72	37.9	6.7	1.71	38.1	6.8	2.70	42.4	1.20	4.4	4.1	211	211	211	181	204	183	-	P
15.2			Plan		,.	NDM / SV	1.68	38.2	8.8	1.71	40.2	5.9	1.69	39.2	7.3	2.70	37.3	1.23	8.4	4.5	181	169	166	133	162	182	-	P
16.1	VIVE	17/04/2019	Fill Area-See Site	~1.2m Placed	Clay SILT w gravels	NDM / SV	1.73	35.7	7.4	1.75	35.9	6.1	1.74	35.8	6.7	2.70	33.6	1.30	8.1	4.9	178	181	175	142	169	181	-	P
16.2			Plan			NDM / SV	1.65	47.5	5.6	1.66	43.7	6.9	1.65	45.6	6.2	2.70	56.4	1.06	1.3	4.4	211	184	211	211	204	183	-	P
17.1	VIVE	26/04/2019	South Retaining	~1.5m below top of RW	Clay SILT	NDM / SV	1.67	45.1	5.6	1.66	47.5	4.6	1.67	46.3	5.1	2.70	49.3	1.12	3.6	4.7	136	157	136	133	141	178	-	P
17.2			Wall Fill	~1m below top of RW		NDM / SV	1.68	48.7	3.4	1.66	50.5	3.4	1.67	49.6	3.4	2.70	55.3	1.07	0.8	3.1	166	208	136	181	173	176	-	P
18.1	VIVE	1/05/2019	South Retaining Wall Fill	~1m below top of RW	Clay SILT	NDM / SV	1.68	43.9	5.5	1.68	46.8	4.3	1.68	45.4	4.9	2.70	52.3	1.10	1.6	3.3	154	142	181	154	158	173	-	P
18.2			Wall I III	1744		NDM / SV		21.3	10.0	1.89	20.9	9.4	1.88	21.1	9.7	2.70	20.9	1.56	9.9	4.3	211	211	181	211	204	175	-	P
19.1	JRA	3/05/2019	South Retaining Wall Fill	~0.5m below FL	Clay Strippings	NDM / SV	1.97	18.0	8.1	1.99	17.6	7.8	1.98	17.8	7.9	2.70	16.8	1.69	8.8	4.9	UTP	UTP	UTP	UTP	UTP	175	-	P
19.2						NDM / SV	1.91	19.1	9.9	1.92	18.3	10.3	1.91	18.7	10.1	2.70	20.9	1.58	8.2	5.5	UTP	UTP	UTP	UTP	UTP	177	-	P
20.1	JRA	8/05/2019	South Retaining Wall Fill	Top of RW (~FL)	Clay Strippings	NDM / SV	1.88	28.7	4.2	1.88	27.6	4.8	1.88	28.2	4.5	2.70	38.3	1.36	0.0	5.1	185	UTP	UTP	UTP	185	174	-	P
20.2			North East Fill - See Site Plan	~1m placed	Clay SILT	NDM / SV	1.76	31.9	7.9	1.77	32.9	7.0	1.77	32.4	7.4	2.70	33.9	1.32	6.5	4.9	185	185	UTP	185	185	177	-	P Retest of URN 11.2

Our Ref - 1009213.0070.0.0/1

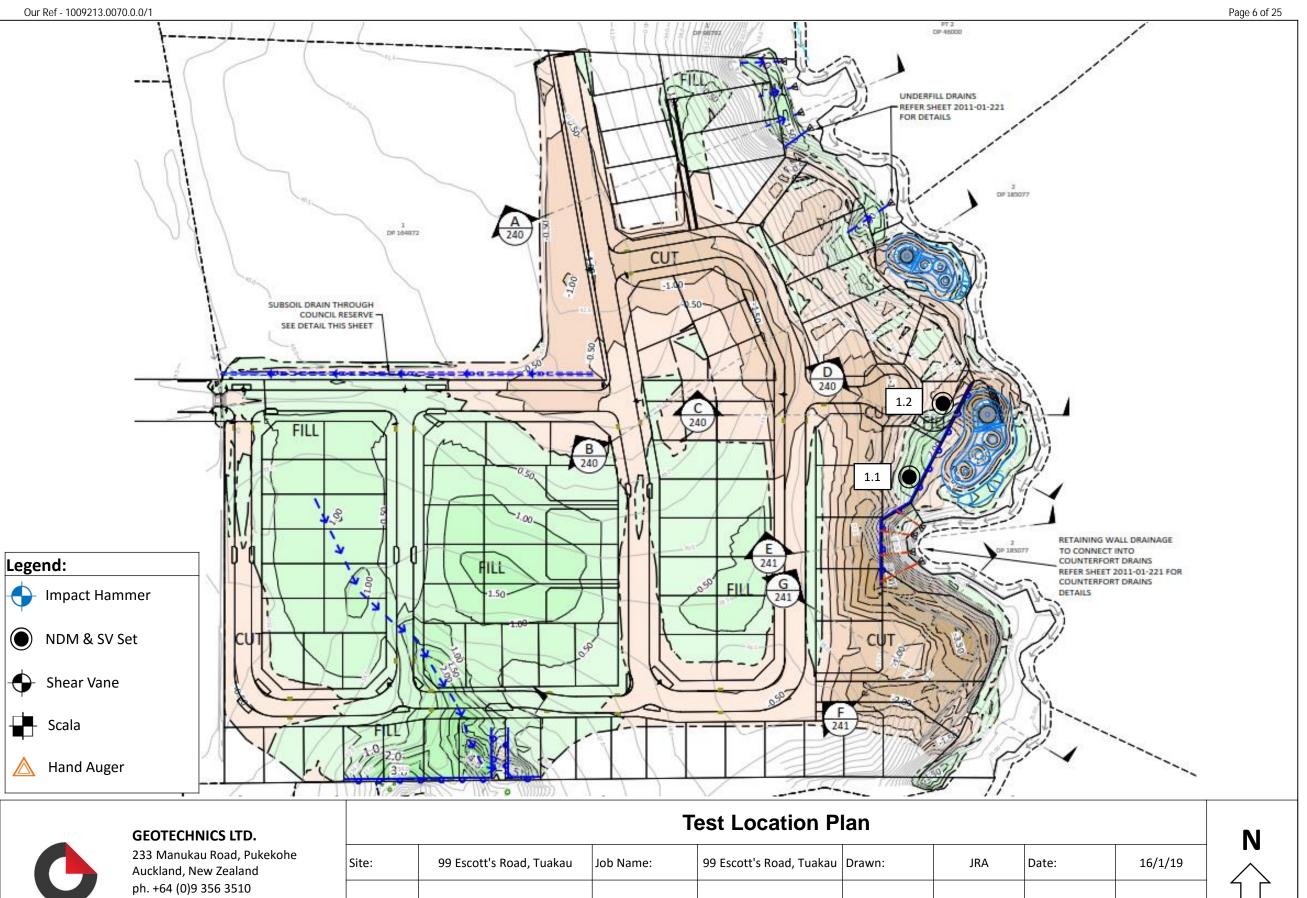


Job: 99 Escotts Road, Tuakau

Customer: Lander Geotechnical Ltd.

Job#	1009213.0070.0.0/1
Entered By	VIVE
Checked By	SEBA
Approved By	JRA

											Nuclear De	nsity (Backsca	tter)					PASS / FAIL				
URN	Tech.	Date	Material Type	Location	Layer	Chainage	Offset	Wet Density					Solid Density (t/m³)	% Solid Density	% Total Voids	Impact Value 1	Retest URN	(P) Pass (F) Fail	Comments			
14.1						-	1	-	-	-	-	-	-	-	-	16	-	Р				
14.2						-	-	-	-	-	-	-	-	-	-	16	-	P				
14.3	VIVE	13/04/2019	CARCE	Old SILT Pond Area -	Subgrade	-	-	-	-	-	-	-	-	-	-	29	-	P	Investigation of Francisco described for health as informed by Lady (Francisco)			
14.4		13/04/2019	GAP 65	See Site Plan	Improvement	=.	-	-	-	-	-	-	-	-	-	20	-	P	Impact value > 15 required for backfill as informed by Jock (Twomey).			
14.5						-	-	-	-	-	-	-	-	-	-	27	-	Р				
14.6						-	1	-	-	-	-	-	-	-	-	18	-	P	<u> </u>			





e. enquiry@geotechnics.co.nz

	Test Location Plan													
Site:	99 Escott's Road, Tuakau	Job Name:	99 Escott's Road, Tuakau	Drawn:	JRA	Date:	16/1/19							
Location:	Retaining Wall Backfill	Job No.:	1009213.0070.0.0/1	URN:	1	Date:	16/1/19							
		Lab Ref:	- N/A	Scale:	Not to Scale	Rev.:	1							





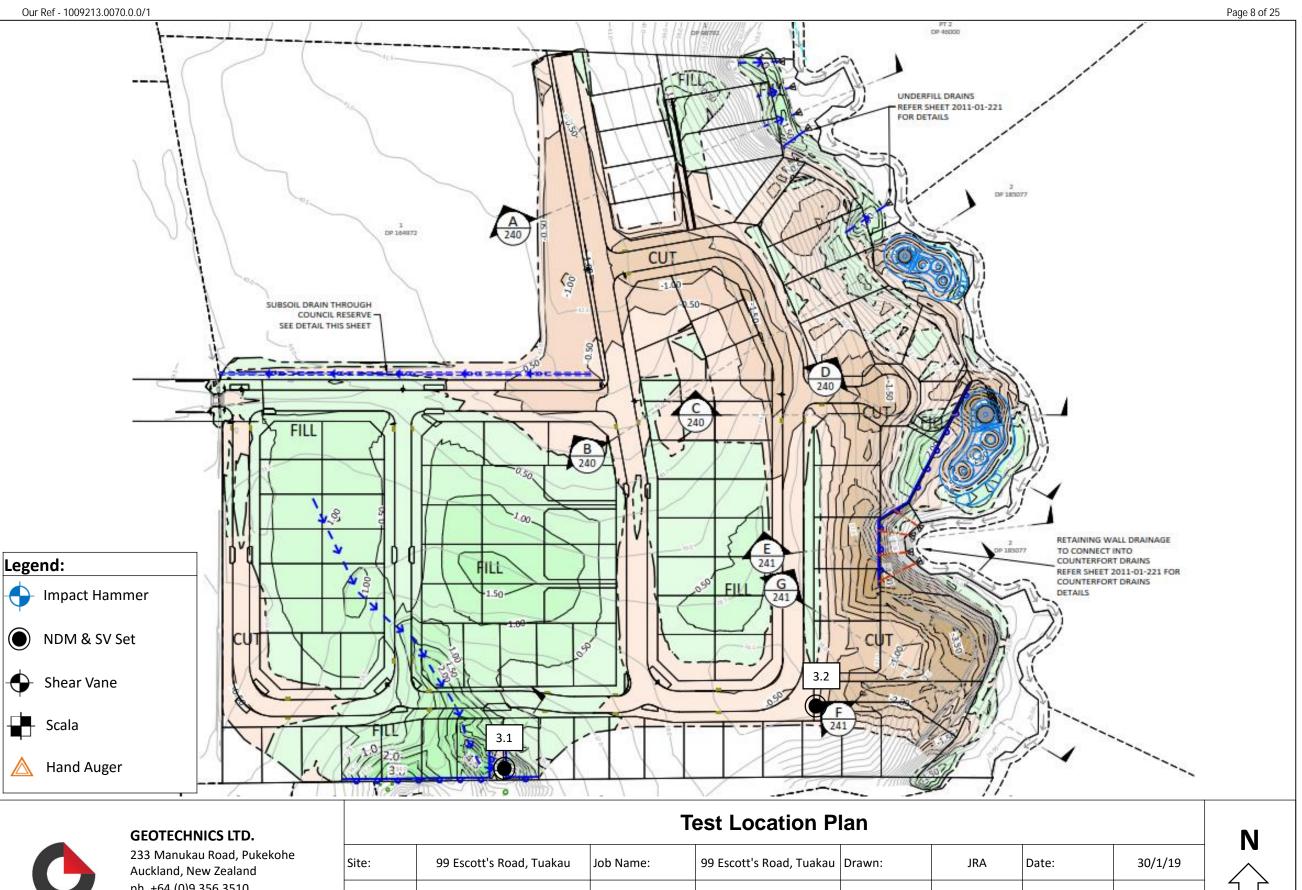


ph. +64 (0)9 356 3510

e. enquiry@geotechnics.co.nz

	Test Location Plan													
Site:	99 Escott's Road, Tuakau	Job Name:	99 Escott's Road, Tuakau	Drawn:	JRA	Date:	24/1/19							
Location:	Stage 2- Fill Area	Job No.:	1009213.0070.0.0/1	URN:	2	Date:	24/1/19							
		Lab Ref:	- N/A	Scale:	Not to Scale	Rev.:	1							







ph. +64 (0)9 356 3510

e. enquiry@geotechnics.co.nz

	lest Location Flan													
Site:	99 Escott's Road, Tuakau	Job Name:	99 Escott's Road, Tuakau	Drawn:	JRA	Date:	30/1/19							
Location:	Fill Areas	Job No.:	1009213.0070.0.0/1	URN:	3	Date:	30/1/19							
		Lab Ref:	- N/A	Scale:	Not to Scale	Rev.:	1							







ph. +64 (0)9 356 3510

e. enquiry@geotechnics.co.nz

	Test Location Plan													
Site:	99 Escott's Road, Tuakau	Job Name:	99 Escott's Road, Tuakau	Drawn:	JRA	Date:	1/2/19							
Location:	South RW Fill Area	Job No.:	1009213.0070.0.0/1	URN:	4	Date:	1/2/19							
		Lab Ref:	- N/A	Scale:	Not to Scale	Rev.:	1							



Our Ref - 1009213.0070.0.0/1 Page 10 of 25 PT 2 DP 46000 UNDERFILL DRAINS
REFER SHEET 2011-01-221
FOR DETAILS 1 DP 164972 (007-SUBSOIL DRAIN THROUGH COUNCIL RESERVE SEE DETAIL THIS SHEET RETAINING WALL DRAINAGE TO CONNECT INTO COUNTERFORT DRAINS Legend: REFER SHEET 2011-01-221 FOR Impact Hammer NDM & SV Set Shear Vane Scala Mand Auger **GEOTECHNICS LTD.**



233 Manukau Road, Pukekohe Auckland, New Zealand ph. +64 (0)9 356 3510

e. enquiry@geotechnics.co.nz

	Test Location Plan													
Site:	99 Escott's Road, Tuakau	Job Name:	99 Escott's Road, Tuakau	Drawn:	JRA	Date:	9/2/19							
Location:	South RW Fill Area	Job No.:	1009213.0070.0.0/1	URN:	5	Date:	9/2/19							
		Lab Ref:	- N/A	Scale:	Not to Scale	Rev.:	1							



Our Ref - 1009213.0070.0.0/1 Page 11 of 25 PT 2 DP 46000 UNDERFILL DRAINS
REFER SHEET 2011-01-221
FOR DETAILS 1 DP 164972 (007-SUBSOIL DRAIN THROUGH COUNCIL RESERVE SEE DETAIL THIS SHEET RETAINING WALL DRAINAGE TO CONNECT INTO COUNTERFORT DRAINS Legend: REFER SHEET 2011-01-221 FOR Impact Hammer NDM & SV Set Shear Vane Scala Scala Mand Auger **GEOTECHNICS LTD.**



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e. enquiry@geotechnics.co.nz

	Test Location Plan													
Site:	99 Escott's Road, Tuakau	Job Name:	99 Escott's Road, Tuakau	Drawn:	JRA	Date:	2/3/19							
Location:	Fill Area	Job No.:	1009213.0070.0.0/1	URN:	6	Date:	2/3/19							
		Lab Ref:	- N/A	Scale:	Not to Scale	Rev.:	1							



Our Ref - 1009213.0070.0.0/1 Page 12 of 25 PT 2 DP 46000 UNDERFILL DRAINS
REFER SHEET 2011-01-221
FOR DETAILS 1 DP 164972 (007-SUBSOIL DRAIN THROUGH COUNCIL RESERVE SEE DETAIL THIS SHEET RETAINING WALL DRAINAGE TO CONNECT INTO COUNTERFORT DRAINS Legend: REFER SHEET 2011-01-221 FOR Impact Hammer NDM & SV Set Shear Vane Scala Mand Auger **GEOTECHNICS LTD.**



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e. enquiry@geotechnics.co.nz

	Test Location Plan													
Site:	99 Escott's Road, Tuakau	Job Name:	99 Escott's Road, Tuakau	Drawn:	JRA	Date:	8/3/19							
Location:	South RW Fill Area	Job No.:	1009213.0070.0.0/1	URN:	7	Date:	8/3/19							
		Lab Ref:	- N/A	Scale:	Not to Scale	Rev.:	1							



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REFER SHEET 2011-01-221
FOR DETAILS 1 DP 164972 (007-SUBSOIL DRAIN THROUGH COUNCIL RESERVE SEE DETAIL THIS SHEET RETAINING WALL DRAINAGE TO CONNECT INTO COUNTERFORT DRAINS Legend: REFER SHEET 2011-01-221 FOR Impact Hammer NDM & SV Set Shear Vane Scala Mand Auger **GEOTECHNICS LTD.** 233 Manukau Road, Pukekohe



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e. enquiry@geotechnics.co.nz

	Test Location Plan													
Site:	99 Escott's Road, Tuakau	Job Name:	99 Escott's Road, Tuakau	Drawn:	JRA	Date:	13/3/19							
Location:	South RW Fill Area	Job No.:	1009213.0070.0.0/1	URN:	8	Date:	13/3/19							
		Lab Ref:	- N/A	Scale:	Not to Scale	Rev.:	1							



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REFER SHEET 2011-01-221
FOR DETAILS 1 DP 164972 (007-SUBSOIL DRAIN THROUGH COUNCIL RESERVE SEE DETAIL THIS SHEET RETAINING WALL DRAINAGE TO CONNECT INTO COUNTERFORT DRAINS Legend: REFER SHEET 2011-01-221 FOR Impact Hammer NDM & SV Set Shear Vane Scala Scala Mand Auger **GEOTECHNICS LTD.** 233 Manukau Road, Pukekohe



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	Test Location Plan									
Site:	99 Escott's Road, Tuakau	Job Name:	99 Escott's Road, Tuakau	Drawn:	JRA	Date:	14/3/19			
Location:	South RW Fill Area	Job No.:	1009213.0070.0.0/1	URN:	9	Date:	14/3/19			
		Lab Ref:	- N/A	Scale:	Not to Scale	Rev.:	1			



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REFER SHEET 2011-01-221
FOR DETAILS 10.2 (007-SUBSOIL DRAIN THROUGH COUNCIL RESERVE SEE DETAIL THIS SHEET RETAINING WALL DRAINAGE TO CONNECT INTO COUNTERFORT DRAINS Legend: REFER SHEET 2011-01-221 FOR Impact Hammer NDM & SV Set Shear Vane Scala A Hand Auger Test I ocation Plan **GEOTECHNICS LTD.** 233 Manukau Road, Pukekohe



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	lest Location Plan									
Site:	99 Escott's Road, Tuakau	Job Name:	99 Escott's Road, Tuakau	Drawn:	JRA	Date:	19/3/19			
Location:	Stage 2 Fill Area	Job No.:	1009213.0070.0.0/1	URN:	10	Date:	19/3/19			
		Lab Ref:	- N/A	Scale:	Not to Scale	Rev.:	1			



Our Ref - 1009213.0070.0.0/1 Page 16 of 25 PT 2 DP 46000 11.1 UNDERFILL DRAINS
REFER SHEET 2011-01-221
FOR DETAILS 1 DP 164972 (007-SUBSOIL DRAIN THROUGH COUNCIL RESERVE SEE DETAIL THIS SHEET 240 RETAINING WALL DRAINAGE TO CONNECT INTO COUNTERFORT DRAINS Legend: REFER SHEET 2011-01-221 FOR Impact Hammer NDM & SV Set Shear Vane Scala Mand Auger **GEOTECHNICS LTD.** 233 Manukau Road, Pukekohe



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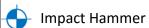
e. enquiry@geotechnics.co.nz

	Test Location Plan								
Site:	99 Escott's Road, Tuakau	Job Name:	99 Escott's Road, Tuakau	Drawn:	VIVE	Date:	27/3/19		
Location:	Stage 2 Fill Area	Job No.:	1009213.0070.0.0/1	URN:	11	Date:	27/3/19		
		Lab Ref:	- N/A	Scale:	Not to Scale	Rev.:	1		



Our Ref - 1009213.0070.0.0/1 Page 17 of 25 PT 2 DP 46000 UNDERFILL DRAINS
REFER SHEET 2011-01-221
FOR DETAILS 1 DP 164972 (007-SUBSOIL DRAIN THROUGH COUNCIL RESERVE SEE DETAIL THIS SHEET















Mand Auger

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	Test Location Plan									
Site:	99 Escott's Road, Tuakau	Job Name:	99 Escott's Road, Tuakau	Drawn:	JRA	Date:	5/4/19			
Location:	South RW Fill Area	Job No.:	1009213.0070.0.0/1	URN:	12	Date:	5/4/19			
		Lab Ref:	- N/A	Scale:	Not to Scale	Rev.:	1			



RETAINING WALL DRAINAGE TO CONNECT INTO COUNTERFORT DRAINS

REFER SHEET 2011-01-221 FOR

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REFER SHEET 2011-01-221
FOR DETAILS 1 DP 164972 (007-SUBSOIL DRAIN THROUGH COUNCIL RESERVE SEE DETAIL THIS SHEET RETAINING WALL DRAINAGE TO CONNECT INTO COUNTERFORT DRAINS Legend: REFER SHEET 2011-01-221 FOR Impact Hammer NDM & SV Set Shear Vane Scala Mand Auger **GEOTECHNICS LTD.** 233 Manukau Road, Pukekohe



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	Test Location Plan									
Site:	99 Escott's Road, Tuakau	Job Name:	99 Escott's Road, Tuakau	Drawn:	JRA	Date:	9/4/19			
Location:	South RW Fill Area	Job No.:	1009213.0070.0.0/1	URN:	13	Date:	9/4/19			
		Lab Ref:	- N/A	Scale:	Not to Scale	Rev.:	1			



Our Ref - 1009213.0070.0.0/1 Page 19 of 25 PT 2 DP 46000 UNDERFILL DRAINS
REFER SHEET 2011-01-221
FOR DETAILS 1 DP 164972 (007-SUBSOIL DRAIN THROUGH COUNCIL RESERVE SEE DETAIL THIS SHEET RETAINING WALL DRAINAGE TO CONNECT INTO COUNTERFORT DRAINS Legend: REFER SHEET 2011-01-221 FOR Impact Hammer NDM & SV Set Shear Vane Scala 14.1-14.6 Mand Auger **GEOTECHNICS LTD.** 233 Manukau Road, Pukekohe



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	Test Location Plan									
Site:	99 Escott's Road, Tuakau	Job Name:	99 Escott's Road, Tuakau	Drawn:	VIVE	Date:	13/4/19			
Location:	Subgrade Improvement @ Detention Tank Area	Job No.:	1009213.0070.0.0/1	URN:	14	Date:	13/4/19			
		Lab Ref:	- N/A	Scale:	Not to Scale	Rev.:	1			



Our Ref - 1009213.0070.0.0/1 Page 20 of 25 PT 2 DP 46000 UNDERFILL DRAINS
REFER SHEET 2011-01-221
FOR DETAILS 1 DP 164972 (007-SUBSOIL DRAIN THROUGH COUNCIL RESERVE SEE DETAIL THIS SHEET 240 RETAINING WALL DRAINAGE TO CONNECT INTO COUNTERFORT DRAINS Legend: REFER SHEET 2011-01-221 FOR Impact Hammer NDM & SV Set Shear Vane Scala Scala Mand Auger **GEOTECHNICS LTD.** 233 Manukau Road, Pukekohe



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	Test Location Plan										
Site:	99 Escott's Road, Tuakau	Job Name:	99 Escott's Road, Tuakau	Drawn:	VIVE	Date:	16/4/19				
Location:	Stage 2 Fill Area	Job No.:	1009213.0070.0.0/1	URN:	15	Date:	16/4/19				
		Lab Ref:	- N/A	Scale:	Not to Scale	Rev.:	1				



Our Ref - 1009213.0070.0.0/1 Page 21 of 25 PT 2 DP 46000 UNDERFILL DRAINS
REFER SHEET 2011-01-221
FOR DETAILS 1 DP 164972 (007-SUBSOIL DRAIN THROUGH COUNCIL RESERVE SEE DETAIL THIS SHEET RETAINING WALL DRAINAGE TO CONNECT INTO COUNTERFORT DRAINS Legend: REFER SHEET 2011-01-221 FOR Impact Hammer NDM & SV Set Shear Vane Scala Mand Auger **GEOTECHNICS LTD.** 233 Manukau Road, Pukekohe



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e. enquiry@geotechnics.co.nz

	Test Location Plan										
Site:	99 Escott's Road, Tuakau	Job Name:	99 Escott's Road, Tuakau	Drawn:	VIVE	Date:	17/4/19				
Location:	Stage 2 Fill Area	Job No.:	1009213.0070.0.0/1	URN:	16	Date:	17/4/19				
		Lab Ref:	- N/A	Scale:	Not to Scale	Rev.:	1				







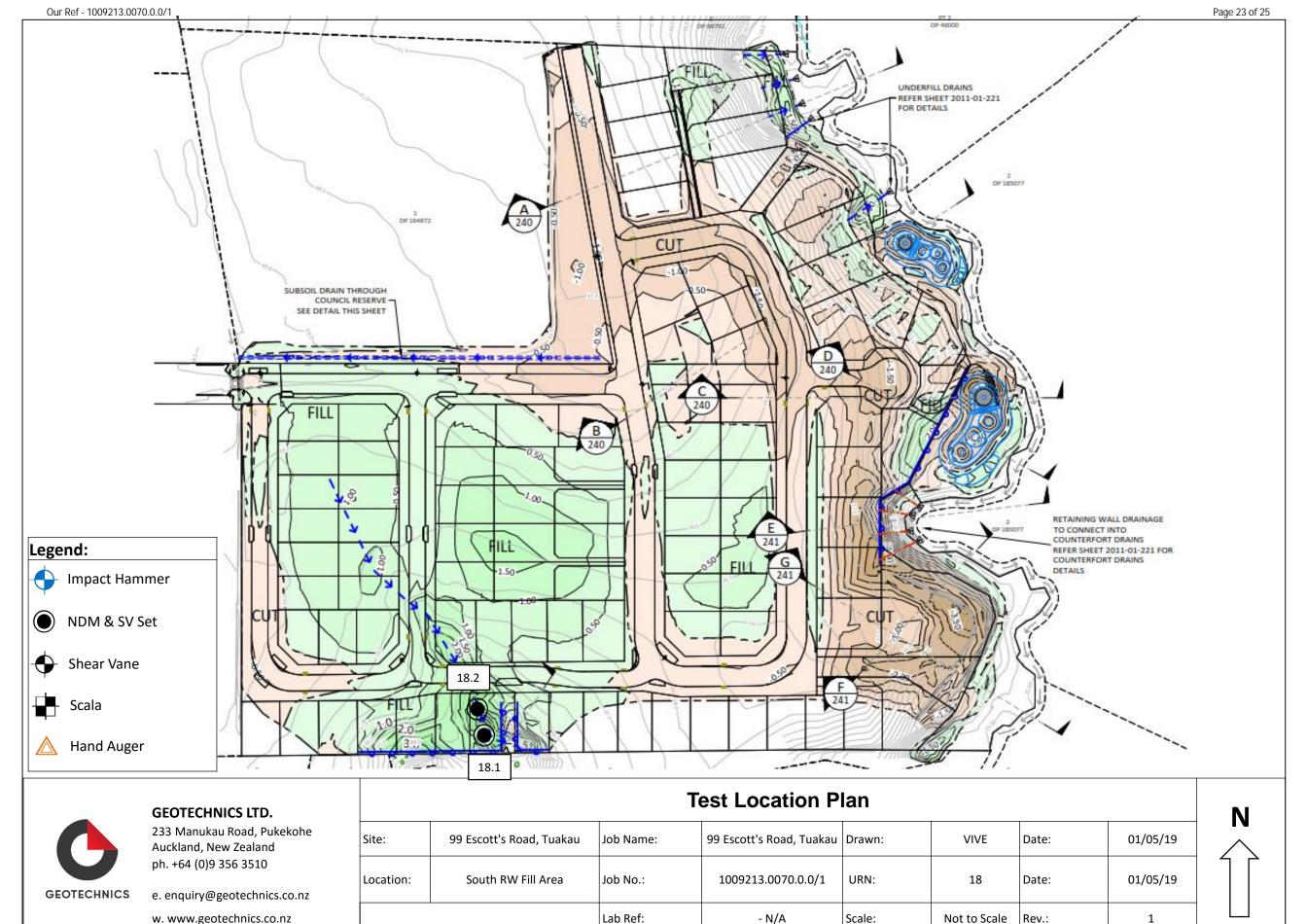
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e. enquiry@geotechnics.co.nz

	Test Location Plan									
Site:	99 Escott's Road, Tuakau	Job Name:	99 Escott's Road, Tuakau	Drawn:	VIVE	Date:	26/4/19			
Location:	South RW Fill Area	Job No.:	1009213.0070.0.0/1	URN:	17	Date:	26/4/19			
		Lab Ref:	- N/A	Scale:	Not to Scale	Rev.:	1			









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	Test Location Plan									
Site:	99 Escott's Road, Tuakau	Job Name:	99 Escott's Road, Tuakau	Drawn:	JRA	Date:	3/05/19			
Location:	South RW Fill Area	Job No.:	1009213.0070.0.0/1	URN:	19	Date:	3/05/19			
		Lab Ref:	- N/A	Scale:	Not to Scale	Rev.:	1			







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	Test Location Plan										
Site:	99 Escott's Road, Tuakau	Job Name:	99 Escott's Road, Tuakau	Drawn:	JRA	Date:	8/05/19				
Location:	South RW Fill Area & Stage 2 North Fill	Job No.:	1009213.0070.0.0/1	URN:	20	Date:	8/05/19				
		Lab Ref:	- N/A	Scale:	Not to Scale	Rev.:	1				

